



January 8, 2021

Mr. Daniel Brown
RaceTrac Petroleum, Inc.
200 Galleria Parkway SE
Atlanta, GA 30339

Reference: Addendum Report of Geotechnical Exploration
RaceTrac Travel Center - Anvil Block
Ellenwood, Georgia

ECS Project No. 10:10706-A

Dear Mr. Brown:

As you are aware, ECS previously submitted a *Report of Geotechnical Exploration* dated November 25, 2020 (ECS Project No. 10:10706). Several of the borings encountered shallow auger refusal within the undocumented fill soils. Test pits performed following the borings encountered trash debris laden soils within parts of the planned development area. ECS was requested to perform additional borings and test pits to evaluate the extent of the debris/rubble laden undocumented fill materials. In addition, ECS was requested to perform several deep borings to auger refusal and hand auger borings with WDCP testing for future planned traffic signal poles at the Anvil Block Road and Tanner Church Road intersection.

The supplemental study was completed in general accordance with ECS Proposal Nos. 10:15641 and 10:15698 dated December 2, 2020 and December 14, 2020, respectively. This addendum should be used in conjunction with ECS's *Report of Geotechnical Exploration* dated November 25, 2020 (ECS Project No. 10:10706) and not as a separate document.

PROJECT INFORMATION

The site is located at the northeast quadrant of the intersection of Anvil Block Road and Tanners Church Road in Ellenwood, Georgia. A Site Location Diagram is included in the Appendix as Figure 1. At the time of fieldwork, the project site was mainly overgrown with shrubs.

The proposed new site development includes the construction of a 1-story convenience store, underground fuel tanks, two pump island canopies with fuel dispensers, underground utilities, and paved parking and driveway areas. We anticipate the building structure will have a combination of concrete masonry shear walls (load bearing) and steel columns supporting roof loads by means of wooden trusses or steel joists. Several storm sewer lines are planned across the site.

The store will be a lightly loaded single story structure with a plan area of approximately 8,100 square feet. Based on our experience with similar RaceTrac projects, we understand the maximum column

loads will not exceed 40 kips, maximum strip loads will not exceed 2.5 kips per linear foot, and maximum floor slab loads will not exceed 150 psf.

Existing grades across the site (convenient store and fuel islands) range from about 875 to 893 feet. Based on the grading plan provided, minimal cuts will be required in parts of the site to achieve the proposed grades. Fills of up to 12 feet will be required at the low-lying area along the site's western boundary adjacent to Tanners Church Road.

The proposed project includes the installation of traffic signal poles at the Anvil Block Road and Tanners Church Road intersection. Based on our experience with previous RaceTrac projects, we understand that the poles are typically supported by shallow foundations. The depth and size of the signal pole foundations will be designed by others. The exact type of pole foundations at the subject site has not yet been determined, but it is our understanding that the poles will be steel strain poles. We understand that steel strain poles foundations are typically drilled shaft (typically 3 feet diameter and no more than 15 feet deep) poured in place with embedded anchor bolts to attach to the base of the pole.

The attached Boring and Test Pit Location Plan (Figure 2) presents the proposed grading information and the site development concept at the time of this report. The Hand Auger and WDCP Test Location Plan (Figure 4) presents the planned intersection improvements at the Anvil Block Road and Tanners Church Road intersection at the time of this report. If any of the information presented is incorrect or has changed, please advise ECS so that we may reevaluate our recommendations considering changes in the present project concept.

FIELD EXPLORATION

Additional Test Borings

To supplement our previous soil test boring and test pit study, ECS performed a total of eleven (11) additional soil test borings designated B-19 through B-29. Boring locations were determined in the field by our representative who used a handheld GPS device. As these methods are not precise, the boring locations shown on the attached Boring and Test Pit Location Plan (Figure 2) should be considered approximate.

The soil test borings were performed with an ATV mounted drill rig, which utilized hollow stem augers to advance the boreholes. No water or drilling fluid was introduced during the process. Representative soil samples were obtained by means of the split-barrel sampling procedure in general accordance with ASTM Specification D-1586 with an automatic drive hammer. In this procedure, a 2-inch O.D., split-barrel sampler is driven into the soil a distance of 18 inches by a 140-pound hammer falling 30 inches.

The number of blows required to drive the sampler through a 12-inch interval is termed the Standard Penetration Test (SPT) N-value and is indicated for each sample on the boring logs. This value can be used as a qualitative indication of the in-place relative density of cohesionless soils. In a less reliable way, it also indicates the consistency of cohesive soils.

The drill crew prepared a field log of the soils encountered in the borings. After recovery, each sample was removed from the sampler and visually classified by the field crew. Representative portions of each sample were then sealed and brought to our laboratory in Marietta, Georgia for further visual examination and laboratory testing by ECS.

Test Pits

To supplement the initial soil test borings and test pits, a total of five (5) additional test pits designated TP-11 through TP-15 were performed. The test pits were excavated to depths ranging from 10 to 14 feet below the ground surface.

The test pits were performed near or between the previous boring and test pit locations. The test pit locations were determined in the field by an ECS representative, referencing measured distances from the prior boring locations and utilizing a hand held GPS unit. During the field work the ECS representative logged the excavated materials and took photographs of the excavations. After completion, the test pit excavations were backfilled.

The test pits locations shown on the attached Boring and Test Pit Location Plan (Figure 2) should be considered approximate. During the test pit excavations representative soil samples were collected and brought to our laboratory in Marietta, Georgia for further visual examination by ECS.

Hand Auger Borings and WDCP Tests

To explore the subsurface conditions at the planned traffic signal borings at the Anvil Block Road and Tanners Church Road intersection, a total of 3 hand auger borings (P-1, P-2, and P-3) were performed. A total of 3 WDCP tests were performed adjacent to the hand auger borings to depths ranging from 9'-2" to 20'-8".

The hand auger and WDCP boring locations were determined in the field by our representative who measured distances and estimated right angles from existing site features. As these methods are not precise, the hand auger boring locations shown on the attached Hand Auger and WDCP Test Location Plan (Figure 4) should be considered approximate.

Representative soil samples for hand auger borings were obtained by means of the hand operated auger sampling procedure in general accordance with ASTM Specification D-1452. In this procedure, the auger boring was made by rotating and advancing the auger bucket to the desired depths while periodically removing the bucket from the hole to clear and examine the auger cuttings.

WDCP testing was conducted adjacent to the hand auger borings to provide relative bearing values at regular intervals throughout the soil profile. In WDCP testing, a cone with a diameter of 1.47 inches is driven into the soil by a 34.94-pound hammer falling 15 inches. The number of blows required to drive the cone through 10 centimeter intervals is recorded. The blows obtained from WDCP can be correlated to Standard Penetration Test (SPT) N-values. Soil samples were not collected during WDCP testing.

Soil Classification

An ECS representative classified each soil sample on the basis of texture and plasticity in accordance with the Unified Soil Classification System. The group symbols for each soil type are indicated in parentheses followed by the soil descriptions on the test pit logs. The various soil types were grouped into the major zones noted on the logs. The stratification lines designating the interfaces between earth materials on the test pit logs are approximate; in-situ, the transitions may be gradual.

The soil samples will be retained in our laboratory for a period of 60 days, after which, they will be discarded unless other instructions are received as to their disposition.

SUBSURFACE CONDITIONS

The subsurface conditions discussed in the following paragraphs and those shown on the test pit logs, boring logs, and hand auger logs represent an estimate of the subsurface conditions based on interpretation of the test pit, boring and hand auger boring data using normally accepted geotechnical engineering judgments. We note that the transition between different soil strata is usually less distinct than those shown on the test pit logs, boring logs, and hand auger boring logs.

Topsoil

Topsoil is a dark-colored surficial material with a high organic content and is generally unsuitable for structural support. Approximately 2 to 6 inches of topsoil was observed in the borings and test pits performed. Observed topsoil thicknesses do not include root mats which could be significantly thicker/deeper and also be unsuitable for structural support. Variation in thicknesses should be expected across the site.

Fill Materials

Fill may be any material that has been transported and deposited by man. Undocumented fill is considered any man placed materials with no moisture-density records from the time it was originally placed.

Materials described as undocumented fill were encountered in all the additional borings and test pits. Shallow auger refusal was encountered within the undocumented fill materials in several of the additional borings performed. The fill materials generally consisted of silty Sand (SM), clayey sand (SC), sandy Silt (ML), and sandy lean Clay (CL).

Figure 3 presents the approximate limits and estimated depth range of the undocumented debris laden fill. Actual extent (horizontal and vertical) of the undocumented fill is expected to be highly variable.

Residual Soils

Residual soil, formed by in-place weathering of the parent rock, was encountered in borings B-19, B-20, B-22, B-23, B-24, B-25, B-26, and B-29 below the ground surface and/or fill materials. Residual soils were encountered below the undocumented fill materials at Test Pits TP-11, TP-12, and TP-15. The residual soil was generally described as firm to stiff sandy Silt (ML), loose clayey Sand (SC), loose to medium dense silty Sand (SM), and stiff sandy fat Clay (CH). N-Values ranged from 2 to 27 bpf.

Groundwater Conditions

Observations for groundwater were made during sampling and upon completion of the drilling operations at each boring location. In auger drilling operations, the groundwater position can often be determined by observing water flowing into or out of the boreholes. Furthermore, visual observation of the soil samples retrieved during the auger drilling exploration can often be used in evaluating the groundwater conditions. Groundwater seepage was observed at Borings B-20, B-24, B-26 and B-29 and Test Pits TP-13, TP-14, and TP-15 at depths ranging from 6 to 27 feet below the existing ground surface.

Summary of Subsurface Soil Conditions

The following tables summarize the subsurface fill conditions reported in the additional borings and test pits. Please refer to the attached individual test pit logs, boring logs, and hand auger logs for more detailed information.

Table 1 - Summary of Subsurface Fill Conditions – Additional Borings

Boring No.	Approximate Existing Elevation (ft.)	Proposed Finished Grade (ft.)	Approximate Proposed Cut (-) or Fill (+) (ft.)	Approximate Depth to Groundwater (ft.)	Approximate Undocumented Fill Depth (ft.)	Boring termination Depth (ft.)
B-19	876 +/-	886	+10	NE	10	20
B-20	882 +/-	886	+4	12.5	10	25
*B-21	884 +/-	889	+5	NE	20	20
B-22	888 +/-	890	+1	NE	10	15
B-23	901 +/-	899	-2	NE	15	20
B-24	889 +/-	889	0	15	10	30
*B-25	893 +/-	889	-4	NE	15	15
B-26	883 +/-	889	+6	24	8	50
*B-27	887 +/-	888.5	+1.5	NE	10	AR @ 10
*B-28	888 +/-	889	+1	NE	6	AR @ 6
B-29	889 +/-	888	-1	27	12	AR @ 46

Notes: AR - Auger Refusal

NE - Not Encountered

*- Boring Terminated/Auger Refusal Encountered Within Undocumented Fill Materials.

Table 2 - Summary of Subsurface Fill Conditions – Additional Test Pits

Test Pit No.	Approximate Existing Elevation (ft.)	Proposed Finished Grade (ft.)	Approximate Proposed Cut(-) or Fill(+) (ft.)	Approximate Depth to Groundwater (ft.)	Approximate Undocumented Fill Depth (ft.)	Test Pit Termination Depth (ft.)
TP-11	900 +/-	899	-1	NE	1	10
TP-12	899 +/-	892	-7	NE	4.5	10
TP-13	883 +/-	888	+5	10	14	14
TP-14	880 +/-	888	+8	6	14	14
TP-15	893 +/-	889	+6	8	3	14

Notes: NE - Not Encountered

ANALYSIS AND RECOMMENDATIONS

General

The additional test pits and borings performed for this study confirmed the presence of undocumented fill materials within parts of the proposed development area. If very soft soils or pockets of debris, organics, stumps, etc., exist within the fill and are not removed during construction, then localized excessive differential settlements could occur in response to new structural loads and the on-going process of volume change which may still occur in the fill. If such non-uniform settlements occur, then moderate distress could result.

Please refer to ECS's *Report of Geotechnical Exploration* dated November 25, 2020 (ECS Project No. 10:10706) for additional guidance on subgrade preparation and fill placement.

In addition, the deep borings performed (B-26 and B-29) encountered auger refusal or dense PWR at depths ranging from 46 to 50 feet below the existing grades.

At the traffic signal poles, we anticipate that some, if not all, of the existing fill will be removed from the pole foundation area during the initial foundation excavation. However, depending on the actual depth of fill at the pole location and the proposed depth to the bottom of foundations, some fill soils may still be present at foundation bearing elevation. The foundation bearing soils should be evaluated by ECS at the time of construction. Some remedial efforts may be required, particularly if low consistency soils or unsuitable materials are encountered at the foundation bearing elevation. Any unsuitable materials or low consistency soils will have to be over-excavated if found within the shallow foundation excavations during construction. Close quality assurance observation by ECS is required.

Drilled Shafts - Traffic Signal Poles

The depth and size of the drilled shafts are generally pre-determined by others, but are typically 3 to 4 feet in diameter, and perhaps as deep as 15 feet.

WDCP test refusal was encountered between 9 and 11 feet below the ground surface WDCP-1 (P-1) and WDCP-2 (P-2). Mechanically operated drill equipment will likely penetrate past WDCP refusal depths. While we anticipate that the drilling equipment required for signal pole construction will advance past the WDCP refusal depths, it should be noted that if either or both of the WDCP refusals are the result of true bedrock conditions, then refusal conditions could possibly be encountered above the required minimum shaft/pole embedment depth in these two areas.

CLOSING

This addendum has been prepared in accordance with generally accepted geotechnical engineering practice. No warranty is expressed or implied. This addendum is based on the available project information, as well as on the results of the limited exploration. This addendum should be used in conjunction with ECS's *Report of Geotechnical Exploration* dated November 25, 2020 (ECS Project No. 10:10706) and not as a separate document.

This addendum is provided for the exclusive use of RaceTrac Petroleum, Inc and their project specific design team. This addendum is not intended to be used or relied upon in connection with other projects or by other third parties. ECS disclaims liability for any such third party use or reliance without express written permission.

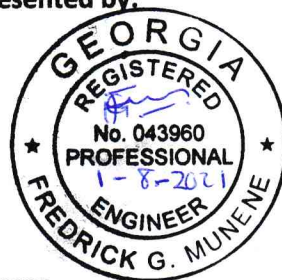
We appreciate the opportunity of working with you on this project and look forward to our continued association. Should you have questions regarding this addendum or need additional consultations, please do not hesitate to contact our office at (770) 590-1971.

Sincerely,

ECS SOUTHEAST, LLP represented by:



Fred Munene, P.E.
Geotechnical Project Manager
GA Registration No. 43960



K. Jay Hornsby P.G.
Principal Geologist
GA P.G. Registration No. 1978

Attachments:

- Figure 1 – Site Location Diagram
- Figure 2 – Boring and Test Pit Location Plan
- Figure 3 – Approximate Limits of Disturbance
- Figure 4 – Hand Auger and WDCP Test Location Plan
- Reference Notes for Boring Logs
- Additional Boring Logs (B-19 through B-29)
- Additional Test Pit Logs (TP-11 through TP-15)
- Test Pit Photo Log
- Hand Auger Logs
- WDCP Logs
- Laboratory Testing Summary
- GBA Important Information about This Geotechnical-Engineering Report



Site Location Diagram

RACETRAC TRAVEL CENTER - ANVIL BLOCK

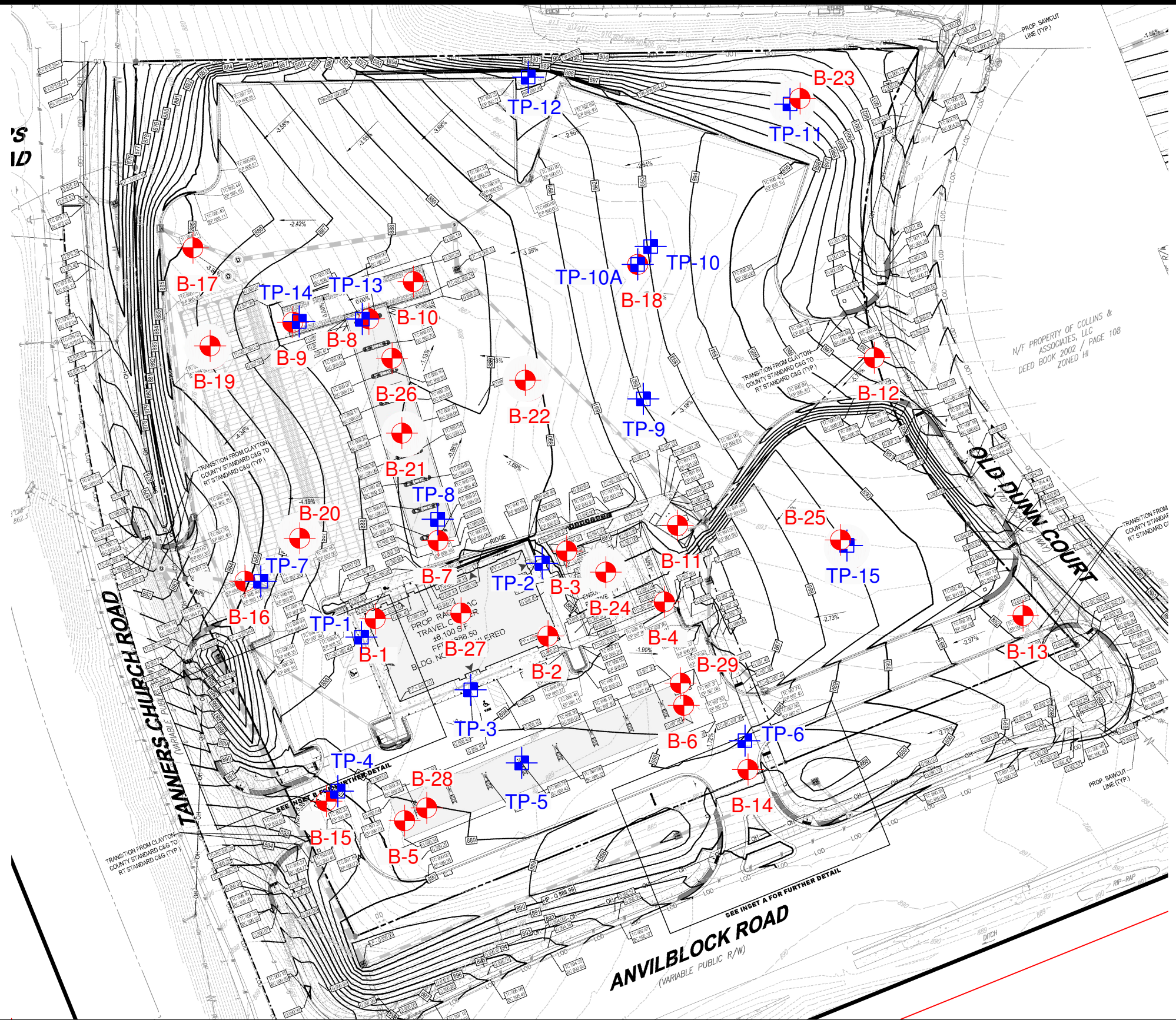
4443 DUNN COURT, ELLENWOOD, GEORGIA

RACETRAC PETROLEUM, INC.SCALE
AS NOTED

PROJECT NO.
10:10706-A

FIGURE
1

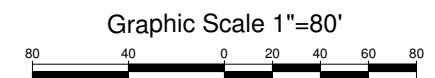
DATE
11/23/2020



LEGEND

-  Approximate Boring Location
B-# Boring Designation

-  Approximate Test Pit Location
TP-# Test Pit Designation



PROJECT: RaceTrac Travel Center - Anvil Block
Ellenwood, GA

PREPARED FOR: RaceTrac Petroleum Inc.

FIGURE NAME: BORING AND TEST PIT LOCATION PLAN

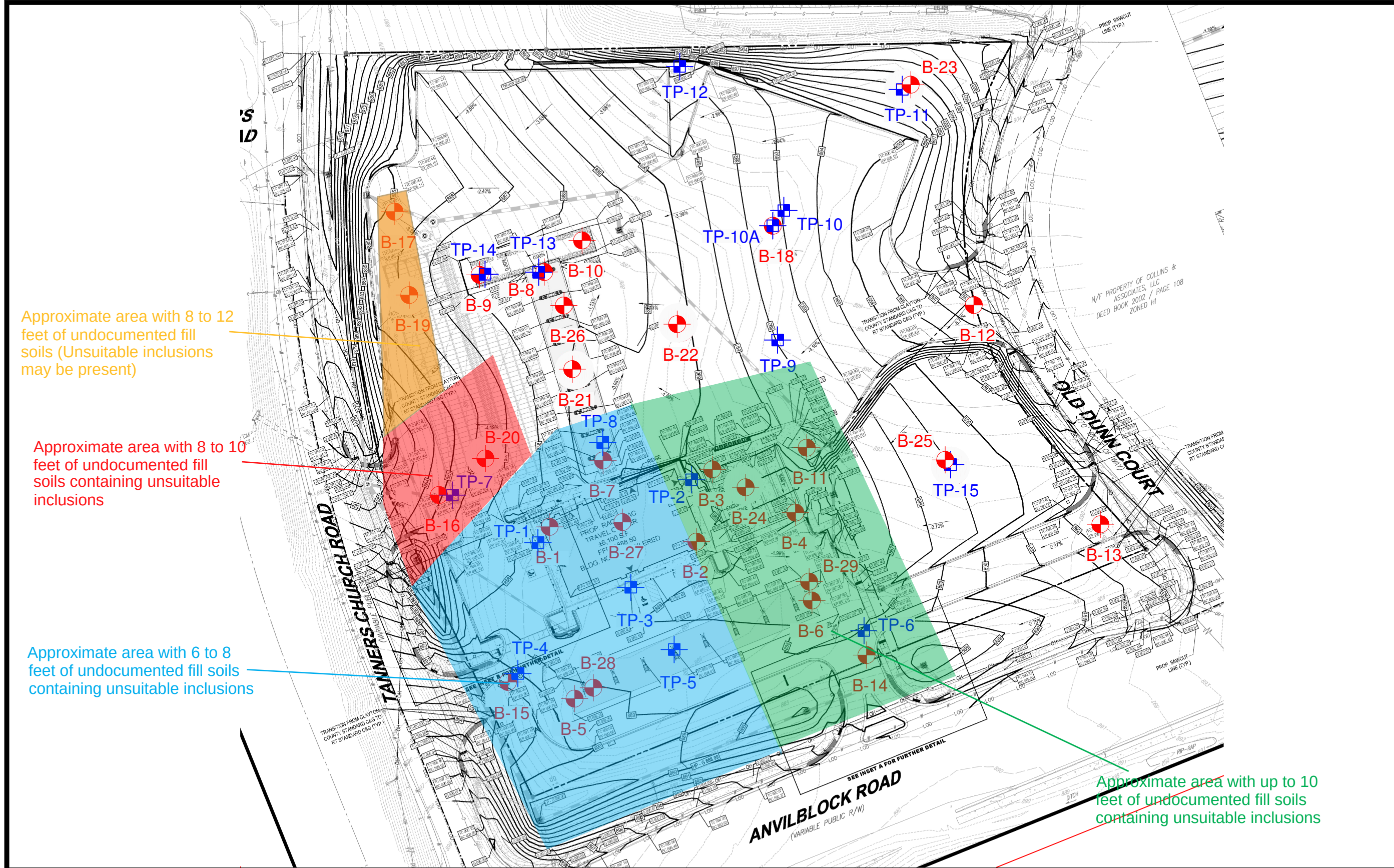
REFERENCE: Grading Plan
RaceTrac Petroleum Inc.
10/09/2020

REVISIONS

JOB NO.	10:10706-A
SCALE	1"=80'
DRAWN	CBS 12/2020
APPR.	KJH 12/2020

Figure No.:

2



LEGEND

-  Approximate Boring Location
-  Approximate Test Pit Location
-  Boring Designation
-  Test Pit Designation



PROJECT: RaceTrac Travel Center - Anvil Block
Ellenwood, GA

PREPARED FOR: RaceTrac Petroleum Inc.

FIGURE NAME: BORING AND TEST PIT LOCATION PLAN

REFERENCE: Grading Plan
RaceTrac Petroleum Inc.
10/09/2020

REVISIONS	
JOB NO.	10:10706-A
SCALE	1"=80'
DRAWN	CBS 12/2020
APPR.	KJH 12/2020

Figure No.:

3



REFERENCE NOTES FOR BORING LOGS

MATERIAL^{1,2}

	ASPHALT
	CONCRETE
	GRAVEL
	TOPSOIL
	VOID
	BRICK
	AGGREGATE BASE COURSE
	GW WELL-GRADED GRAVEL gravel-sand mixtures, little or no fines
	GP POORLY-GRADED GRAVEL gravel-sand mixtures, little or no fines
	GM SILTY GRAVEL gravel-sand-silt mixtures
	GC CLAYEY GRAVEL gravel-sand-clay mixtures
	SW WELL-GRADED SAND gravelly sand, little or no fines
	SP POORLY-GRADED SAND gravelly sand, little or no fines
	SM SM SILTY SAND sand-silt mixtures
	SC CLAYEY SAND sand-clay mixtures
	ML SILT non-plastic to medium plasticity
	MH ELASTIC SILT high plasticity
	CL LEAN CLAY low to medium plasticity
	CH FAT CLAY high plasticity
	OL ORGANIC SILT or CLAY non-plastic to low plasticity
	OH ORGANIC SILT or CLAY high plasticity
	PT PEAT highly organic soils

DRILLING SAMPLING SYMBOLS & ABBREVIATIONS

SS	Split Spoon Sampler	PM	Pressuremeter Test
ST	Shelby Tube Sampler	RD	Rock Bit Drilling
WS	Wash Sample	RC	Rock Core, NX, BX, AX
BS	Bulk Sample of Cuttings	REC	Rock Sample Recovery %
PA	Power Auger (no sample)	RQD	Rock Quality Designation %
HSA	Hollow Stem Auger		

PARTICLE SIZE IDENTIFICATION

DESIGNATION	PARTICLE SIZES
Boulders	12 inches (300 mm) or larger
Cobbles	3 inches to 12 inches (75 mm to 300 mm)
Gravel: Coarse	¾ inch to 3 inches (19 mm to 75 mm)
Fine	4.75 mm to 19 mm (No. 4 sieve to ¾ inch)
Sand: Coarse	2.00 mm to 4.75 mm (No. 10 to No. 4 sieve)
Medium	0.425 mm to 2.00 mm (No. 40 to No. 10 sieve)
Fine	0.074 mm to 0.425 mm (No. 200 to No. 40 sieve)
Silt & Clay ("Fines")	<0.074 mm (smaller than a No. 200 sieve)

COHESIVE SILTS & CLAYS

UNCONFINED COMPRESSIVE STRENGTH, QP ⁴	SPT ⁵ (BPF)	CONSISTENCY ⁷ (COHESIVE)
<0.25	<3	Very Soft
0.25 - <0.50	3 - 4	Soft
0.50 - <1.00	5 - 8	Firm
1.00 - <2.00	9 - 15	Stiff
2.00 - <4.00	16 - 30	Very Stiff
4.00 - 8.00	31 - 50	Hard
>8.00	>50	Very Hard

RELATIVE AMOUNT ⁷	COARSE GRAINED (%) ⁸	FINE GRAINED (%) ⁸
Trace	≤5	≤5
With	10 - 20	10 - 25
Adjective (ex: "Silty")	25 - 45	30 - 45

GRAVELS, SANDS & NON-COHESIVE SILTS

SPT ⁵	DENSITY
<5	Very Loose
5 - 10	Loose
11 - 30	Medium Dense
31 - 50	Dense
>50	Very Dense

WATER LEVELS⁶

	WL (First Encountered)
	WL (Completion)
	WL (Seasonal High Water)
	WL (Stabilized)

FILL AND ROCK

FILL	POSSIBLE FILL	PROBABLE FILL	ROCK

¹Classifications and symbols per ASTM D 2488-17 (Visual-Manual Procedure) unless noted otherwise.

²To be consistent with general practice, "POORLY GRADED" has been removed from GP, GP-GM, GP-GC, SP, SP-SM, SP-SC soil types on the boring logs.

³Non-ASTM designations are included in soil descriptions and symbols along with ASTM symbol [Ex: (SM-FILL)].

⁴Typically estimated via pocket penetrometer or Torvane shear test and expressed in tons per square foot (tsf).

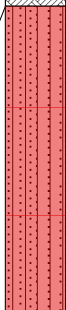
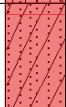
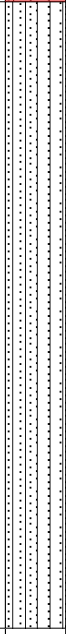
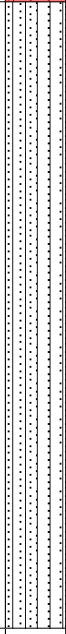
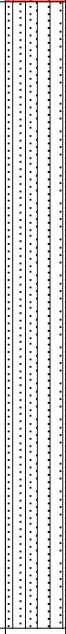
⁵Standard Penetration Test (SPT) refers to the number of hammer blows (blow count) of a 140 lb. hammer falling 30 inches on a 2 inch OD split spoon sampler required to drive the sampler 12 inches (ASTM D 1586). "N-value" is another term for "blow count" and is expressed in blows per foot (bpf). SPT correlations per 7.4.2 Method B and need to be corrected if using an auto hammer.

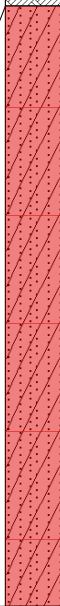
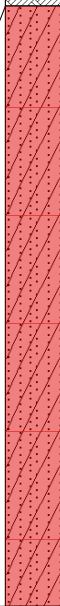
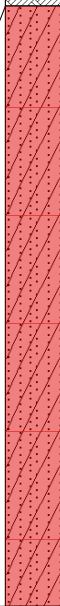
⁶The water levels are those levels actually measured in the borehole at the times indicated by the symbol. The measurements are relatively reliable when augering, without adding fluids, in granular soils. In clay and cohesive silts, the determination of water levels may require several days for the water level to stabilize. In such cases, additional methods of measurement are generally employed.

⁷Minor deviation from ASTM D 2488-17 Note 14.

⁸Percentages are estimated to the nearest 5% per ASTM D 2488-17.

CLIENT: RaceTrac Petroleum, Inc.				PROJECT NO.: 10:10706-B		BORING NO.: B-19		SHEET: 1 of 1		
PROJECT NAME: RaceTrac Travel Center - Anvil Block Additional Borings				DRILLER/CONTRACTOR: Geo Lab, Inc. dba Geo Lab Drilling						
SITE LOCATION: 4443 Dunn Court, Ellenwood, Georgia 30294								LOSS OF CIRCULATION 		
NORTHING:		EASTING:		STATION:		SURFACE ELEVATION: 876 +/-		BOTTOM OF CASING 		
DEPTH (FT)	SAMPLE NUMBER	SAMPLE TYPE	SAMPLE DIST. (IN)	RECOVERY (IN)	DESCRIPTION OF MATERIAL	WATER LEVELS	ELEVATION (FT)	BLOWS/6"	Plastic Limit Water Content Liquid Limit X ————— ● ————— △ ⊗ STANDARD PENETRATION BLOWS/FT ROCK QUALITY DESIGNATION & RECOVERY — RQD — REC ○ CALIBRATED PENETROMETER TON/SF [FINES CONTENT] %	
					Topsoil Thickness[2"]					
	S-1	SS	18	18	(SC FILL) FILL, CLAYEY SAND, gray to dark brown, moist to wet, very loose to loose			1-1-1 (2)	⊗ ₂	
5	S-2	SS	0	0			871			
	S-3	SS	18	18				2-2-2 (4)	⊗ ₄	
10	S-4	SS	18	18			866	2-2-4 (6)	⊗ ₆	
					(SM) SILTY SAND, contains rock fragments, reddish brown to white, moist, loose to medium dense					
15	S-5	SS	18	18			861	4-5-4 (9)	⊗ ₉	
20	S-6	SS	18	18			856	4-5-7 (12)	⊗ ₁₂	
					END OF DRILLING AT 20 FT					
25							851			
30							846			
THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY LINES BETWEEN SOIL TYPES. IN-SITU THE TRANSITION MAY BE GRADUAL										
▽ WL (First Encountered) Dry					BORING STARTED: Dec 10 2020		CAVE IN DEPTH: 6			
▼ WL (Completion)					BORING COMPLETED: Dec 10 2020		HAMMER TYPE: Auto			
▽ WL (Seasonal High Water)					EQUIPMENT: ATV		LOGGED BY: NLC		DRILLING METHOD: HSA	
▽ WL (Stabilized)										
GEOTECHNICAL BOREHOLE LOG										

CLIENT: RaceTrac Petroleum, Inc.				PROJECT NO.: 10:10706-B		BORING NO.: B-20		SHEET: 1 of 1		
PROJECT NAME: RaceTrac Travel Center - Anvil Block Additional Borings				DRILLER/CONTRACTOR: Geo Lab, Inc. dba Geo Lab Drilling						
SITE LOCATION: 4443 Dunn Court, Ellenwood, Georgia 30294										
NORTHING:		EASTING:		STATION:		SURFACE ELEVATION: 882 +/-		LOSS OF CIRCULATION 		
								BOTTOM OF CASING 		
DEPTH (FT)	SAMPLE NUMBER	SAMPLE TYPE	SAMPLE DIST. (IN)	RECOVERY (IN)	DESCRIPTION OF MATERIAL	WATER LEVELS	ELEVATION (FT)	BLOWS/6"	Plastic Limit Water Content Liquid Limit X ————— ● ————— △ ⊗ STANDARD PENETRATION BLOWS/FT ROCK QUALITY DESIGNATION & RECOVERY — RQD — REC ○ CALIBRATED PENETROMETER TON/SF [FINES CONTENT] %	
5	S-1	SS	18	18	Topsoil Thickness[2"] (SM FILL) FILL, SILTY SAND, contains slight mica, reddish brown to dark brown, moist, very loose		877	3-2-1 (3)	⊗ ₃	
	S-2	SS	18	18			877	1-2-2 (4)	⊗ ₄	
	S-3	SS	18	18			877	WOH-WOH-WOH (0)	⊗	
10	S-4	SS	18	18	(SC FILL) FILL, CLAYEY SAND, dark gray, moist, very loose		872	WOH-WOH-WOH (0)	⊗	
					(SM) SILTY SAND, contains significant mica, dark brown to reddish brown, moist, very loose to medium dense		872	WOH-WOH-WOH (0)	⊗	
15	S-5	SS	18	18			867	WOH-WOH-WOH (0)	⊗	
							867			
20	S-6	SS	18	18			862	4-4-4 (8)	⊗ ₈	
							862			
25	S-7	SS	18	18			857	5-6-5 (11)	⊗ ₁₁	
					END OF DRILLING AT 25 FT		857			
30										
THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY LINES BETWEEN SOIL TYPES. IN-SITU THE TRANSITION MAY BE GRADUAL										
∇ WL (First Encountered) 12.5					BORING STARTED: Dec 10 2020		CAVE IN DEPTH: 14			
▼ WL (Completion)					BORING COMPLETED: Dec 10 2020		HAMMER TYPE: Auto			
∇ WL (Seasonal High Water)					EQUIPMENT: ATV		LOGGED BY: NLC		DRILLING METHOD: HSA	
∇ WL (Stabilized)										
GEOTECHNICAL BOREHOLE LOG										

CLIENT: RaceTrac Petroleum, Inc.				PROJECT NO.: 10:10706-B		BORING NO.: B-21		SHEET: 1 of 1																																																																																																																																								
PROJECT NAME: RaceTrac Travel Center - Anvil Block Additional Borings				DRILLER/CONTRACTOR: Geo Lab, Inc. dba Geo Lab Drilling																																																																																																																																												
SITE LOCATION: 4443 Dunn Court, Ellenwood, Georgia 30294								LOSS OF CIRCULATION 																																																																																																																																								
NORTHING:		EASTING:		STATION:		SURFACE ELEVATION: 884 +/-		BOTTOM OF CASING 																																																																																																																																								
<table><thead><tr><th rowspan="4">DEPTH (FT)</th><th rowspan="4">SAMPLE NUMBER</th><th rowspan="4">SAMPLE TYPE</th><th rowspan="4">SAMPLE DIST. (IN)</th><th rowspan="4">RECOVERY (IN)</th><th rowspan="4">DESCRIPTION OF MATERIAL</th><th rowspan="4">WATER LEVELS</th><th rowspan="4">ELEVATION (FT)</th><th rowspan="4">BLOWS/6"</th><th colspan="3">Plastic Limit Water Content Liquid Limit X ————— ● ————— Δ</th></tr><tr><th colspan="3">⊗ STANDARD PENETRATION BLOWS/FT</th></tr><tr><th colspan="3">— RQD</th></tr><tr><th colspan="3">— REC</th></tr><tr><th colspan="9"></th><th colspan="3">○ CALIBRATED PENETROMETER TON/SF [FINES CONTENT] %</th></tr></thead><tbody><tr><td rowspan="5">5</td><td>S-1</td><td>SS</td><td>18</td><td>18</td><td rowspan="5">Topsoil Thickness[2"] (SC FILL) FILL, CLAYEY SAND, gray to tannish brown, moist, very loose to loose</td><td rowspan="5"></td><td rowspan="5">879</td><td>2-1-1 (2)</td><td>⊗₂</td><td colspan="2"></td></tr><tr><td>S-2</td><td>SS</td><td>18</td><td>18</td><td>2-3-6 (9)</td><td>⊗₉</td><td colspan="2"></td></tr><tr><td>S-3</td><td>SS</td><td>18</td><td>18</td><td>3-4-5 (9)</td><td>⊗₉</td><td colspan="2"></td></tr><tr><td>S-4</td><td>SS</td><td>18</td><td>18</td><td>3-3-4 (7)</td><td>⊗₇</td><td colspan="2"></td></tr><tr><td>S-5</td><td>SS</td><td>18</td><td>18</td><td>2-4-3 (7)</td><td>⊗₇</td><td colspan="2"></td></tr><tr><td rowspan="2">15</td><td></td><td></td><td></td><td></td><td rowspan="2">(SM FILL) FILL, SILTY SAND, contains slight mica, reddish brown to dark brown, moist, medium dense</td><td rowspan="2"></td><td rowspan="2">869</td><td></td><td></td><td colspan="2"></td></tr><tr><td>S-6</td><td>SS</td><td>18</td><td>18</td><td>5-7-5 (12)</td><td>⊗₁₂</td><td colspan="2"></td></tr><tr><td>20</td><td colspan="4"></td><td>END OF DRILLING AT 20 FT</td><td></td><td>864</td><td></td><td></td><td colspan="2"></td></tr><tr><td>25</td><td colspan="4"></td><td></td><td></td><td>859</td><td></td><td></td><td colspan="2"></td></tr><tr><td>30</td><td colspan="4"></td><td></td><td></td><td>854</td><td></td><td></td><td colspan="2"></td></tr></tbody></table>												DEPTH (FT)	SAMPLE NUMBER	SAMPLE TYPE	SAMPLE DIST. (IN)	RECOVERY (IN)	DESCRIPTION OF MATERIAL	WATER LEVELS	ELEVATION (FT)	BLOWS/6"	Plastic Limit Water Content Liquid Limit X ————— ● ————— Δ			⊗ STANDARD PENETRATION BLOWS/FT			— RQD			— REC												○ CALIBRATED PENETROMETER TON/SF [FINES CONTENT] %			5	S-1	SS	18	18	Topsoil Thickness[2"] (SC FILL) FILL, CLAYEY SAND, gray to tannish brown, moist, very loose to loose		879	2-1-1 (2)	⊗ ₂			S-2	SS	18	18	2-3-6 (9)	⊗ ₉			S-3	SS	18	18	3-4-5 (9)	⊗ ₉			S-4	SS	18	18	3-3-4 (7)	⊗ ₇			S-5	SS	18	18	2-4-3 (7)	⊗ ₇			15					(SM FILL) FILL, SILTY SAND, contains slight mica, reddish brown to dark brown, moist, medium dense		869					S-6	SS	18	18	5-7-5 (12)	⊗ ₁₂			20					END OF DRILLING AT 20 FT		864					25							859					30							854				
DEPTH (FT)	SAMPLE NUMBER	SAMPLE TYPE	SAMPLE DIST. (IN)	RECOVERY (IN)	DESCRIPTION OF MATERIAL	WATER LEVELS	ELEVATION (FT)	BLOWS/6"	Plastic Limit Water Content Liquid Limit X ————— ● ————— Δ																																																																																																																																							
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	S-2	SS	18	18				2-3-6 (9)	⊗ ₉																																																																																																																																							
	S-3	SS	18	18				3-4-5 (9)	⊗ ₉																																																																																																																																							
	S-4	SS	18	18				3-3-4 (7)	⊗ ₇																																																																																																																																							
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	S-6	SS	18	18				5-7-5 (12)	⊗ ₁₂																																																																																																																																							
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THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY LINES BETWEEN SOIL TYPES. IN-SITU THE TRANSITION MAY BE GRADUAL																																																																																																																																																
▽ WL (First Encountered) 6				BORING STARTED: Dec 10 2020				CAVE IN DEPTH: 17																																																																																																																																								
▼ WL (Completion)				BORING COMPLETED: Dec 10 2020				HAMMER TYPE: Auto																																																																																																																																								
▼ WL (Seasonal High Water)				EQUIPMENT: ATV				LOGGED BY: NLC																																																																																																																																								
▽ WL (Stabilized)								DRILLING METHOD: HSA																																																																																																																																								
GEOTECHNICAL BOREHOLE LOG																																																																																																																																																

CLIENT: RaceTrac Petroleum, Inc.				PROJECT NO.: 10:10706-B		BORING NO.: B-22		SHEET: 1 of 1		
PROJECT NAME: RaceTrac Travel Center - Anvil Block Additional Borings				DRILLER/CONTRACTOR: Geo Lab, Inc. dba Geo Lab Drilling						
SITE LOCATION: 4443 Dunn Court, Ellenwood, Georgia 30294										
NORTHING:		EASTING:		STATION:		SURFACE ELEVATION: 888 +/-		LOSS OF CIRCULATION 		
								BOTTOM OF CASING 		
DEPTH (FT)	SAMPLE NUMBER	SAMPLE TYPE	SAMPLE DIST. (IN)	RECOVERY (IN)	DESCRIPTION OF MATERIAL	WATER LEVELS	ELEVATION (FT)	BLOWS/6"	Plastic Limit Water Content Liquid Limit X ————— ● ————— △	
									⊗ STANDARD PENETRATION BLOWS/FT	
									ROCK QUALITY DESIGNATION & RECOVERY	
									— RQD — REC	
									○ CALIBRATED PENETROMETER TON/SF [FINES CONTENT] %	
					Topsoil Thickness[2"]					
	S-1	SS	18	18	(ML FILL) FILL, SANDY SILT, reddish brown, moist, firm			3-3-5 (8)	⊗ ₈	
5	S-2	SS	18	18	(SC FILL) FILL, CLAYEY SAND, reddish brown to tannish brown, moist, loose to medium dense		883	2-4-6 (10)	⊗ ₁₀	
	S-3	SS	18	18				5-5-6 (11)	⊗ ₁₁	
10	S-4	SS	18	18			878	3-3-4 (7)	⊗ ₇	
					(SM) SILTY SAND, contains rock fragments, tannish brown to white, moist, medium dense					
15	S-5	SS	18	18			873	6-6-5 (11)	⊗ ₁₁	
	END OF DRILLING AT 15 FT									
20							868			
25							863			
30							858			
THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY LINES BETWEEN SOIL TYPES. IN-SITU THE TRANSITION MAY BE GRADUAL										
▽ WL (First Encountered) Dry					BORING STARTED: Dec 10 2020		CAVE IN DEPTH: 11			
▼ WL (Completion)					BORING COMPLETED: Dec 10 2020		HAMMER TYPE: Auto			
▽ WL (Seasonal High Water)					EQUIPMENT: ATV		LOGGED BY: NLC		DRILLING METHOD: HSA	
▽ WL (Stabilized)										
GEOTECHNICAL BOREHOLE LOG										

CLIENT: RaceTrac Petroleum, Inc.				PROJECT NO.: 10:10706-B		BORING NO.: B-23		SHEET: 1 of 1			
PROJECT NAME: RaceTrac Travel Center - Anvil Block Additional Borings				DRILLER/CONTRACTOR: Geo Lab, Inc. dba Geo Lab Drilling							
SITE LOCATION: 4443 Dunn Court, Ellenwood, Georgia 30294											
NORTHING:		EASTING:		STATION:		SURFACE ELEVATION: 901 +/-		LOSS OF CIRCULATION 			
								BOTTOM OF CASING 			
DEPTH (FT)	SAMPLE NUMBER	SAMPLE TYPE	SAMPLE DIST. (IN)	RECOVERY (IN)	DESCRIPTION OF MATERIAL	WATER LEVELS	ELEVATION (FT)	BLOWS/6"	Plastic Limit Water Content Liquid Limit X ————— ● ————— △		
									⊗ STANDARD PENETRATION BLOWS/FT		
									ROCK QUALITY DESIGNATION & RECOVERY		
									— RQD — REC		
									○ CALIBRATED PENETROMETER TON/SF [FINES CONTENT] %		
					Topsoil Thickness[2"]						
	S-1	SS	18	18	(ML FILL) FILL, SANDY SILT, reddish brown, moist, stiff			4-3-6 (9)	⊗ ₉		
5	S-2	SS	18	18			896	3-4-6 (10)	⊗ ₁₀		
					(SM FILL) FILL, SILTY SAND, contains slight mica, reddish brown, moist, loose			3-4-4 (8)	⊗ ₈		
	S-3	SS	18	18				3-5-5 (10)	⊗ ₁₀		
10	S-4	SS	18	18			891	2-2-3 (5)	⊗ ₅		
								4-5-8 (13)	⊗ ₁₃		
15	S-5	SS	18	18	(SM) SILTY SAND, contains mica, reddish brown to dark brown, moist, medium dense		886				
20	S-6	SS	18	18			881				
					END OF DRILLING AT 20 FT						
25							876				
30							871				
THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY LINES BETWEEN SOIL TYPES. IN-SITU THE TRANSITION MAY BE GRADUAL											
☒ WL (First Encountered) Dry				BORING STARTED: Dec 10 2020				CAVE IN DEPTH: 14			
☒ WL (Completion)				BORING COMPLETED: Dec 10 2020				HAMMER TYPE: Auto			
☒ WL (Seasonal High Water)				EQUIPMENT: ATV		LOGGED BY: NLC		DRILLING METHOD: HSA			
☒ WL (Stabilized)											
GEOTECHNICAL BOREHOLE LOG											

CLIENT: RaceTrac Petroleum, Inc.						PROJECT NO.: 10:10706-B		BORING NO.: B-24		SHEET: 1 of 1		
PROJECT NAME: RaceTrac Travel Center - Anvil Block Additional Borings						DRILLER/CONTRACTOR: Geo Lab, Inc. dba Geo Lab Drilling						
SITE LOCATION: 4443 Dunn Court, Ellenwood, Georgia 30294								LOSS OF CIRCULATION ▶		BOTTOM OF CASING ▶		
NORTHING:		EASTING:		STATION:		SURFACE ELEVATION: 889 +/-						
DEPTH (FT)	SAMPLE NUMBER	SAMPLE TYPE	SAMPLE DIST. (IN)	RECOVERY (IN)	DESCRIPTION OF MATERIAL	WATER LEVELS	ELEVATION (FT)	BLOWS/6"	Plastic Limit Water Content Liquid Limit X ————— ● ————— Δ [*] ⊗ STANDARD PENETRATION BLOWS/FT ROCK QUALITY DESIGNATION & RECOVERY — RQD — REC ○ CALIBRATED PENETROMETER TON/SF [FINES CONTENT] %			
5	S-1	SS	18	18	Topsoil Thickness[2"] (SC FILL) FILL, CLAYEY SAND, dark brown, moist, very loose		884	4-1-1 (2)	⊗ ₂			
	S-2	SS	18	18	(SM FILL) FILL, SILTY SAND, contains rock fragments, reddish brown to dark brown, moist to wet, very loose to very dense			1-1-2 (3)	⊗ ₃			
	S-3	SS	10	10				13-50/4" (50/4")	⊗ _{50/4"}			
10	S-4	SS	18	18			879	9-19-18 (37)	⊗ ₃₇			
					(SC) CLAYEY SAND, contains slight mica rock fragments, gray to dark brown, moist, very loose to loose			3-1-1 (2)	⊗ ₂			
15	S-5	SS	18	18			874	1-3-5 (8)	⊗ ₈			
	S-6	SS	18	18				5-4-4 (8)	⊗ ₈			
20					(SM) SILTY SAND, contains significant mica, gray to tannish brown, moist, loose to medium dense		869	6-7-10 (17)	⊗ ₁₇			
	S-7	SS	18	18								
25							864					
	S-8	SS	18	18								
30					END OF DRILLING AT 30 FT		859					
THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY LINES BETWEEN SOIL TYPES. IN-SITU THE TRANSITION MAY BE GRADUAL												
▽ WL (First Encountered)		15		BORING STARTED: Dec 10 2020		CAVE IN DEPTH: 18						
▼ WL (Completion)				BORING COMPLETED: Dec 10 2020		HAMMER TYPE: Auto						
▼ WL (Seasonal High Water)				EQUIPMENT: ATV		LOGGED BY: NLC		DRILLING METHOD: HSA				
▽ WL (Stabilized)												

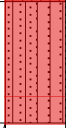
GEOTECHNICAL BOREHOLE LOG

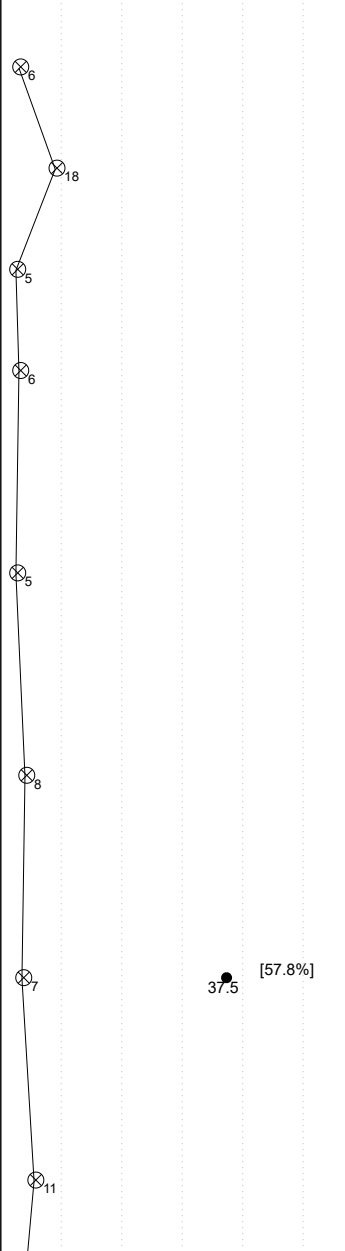
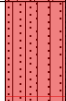
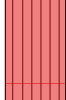
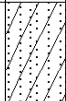
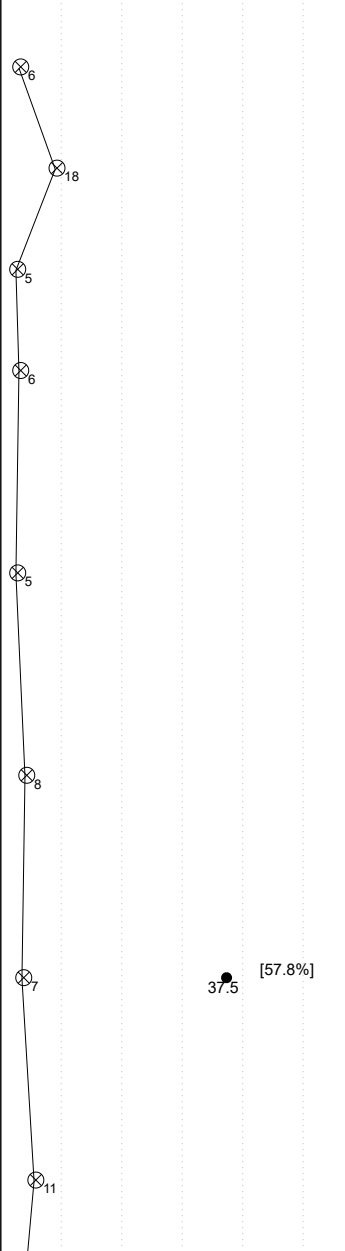
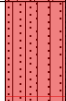
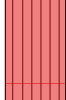
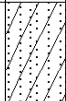
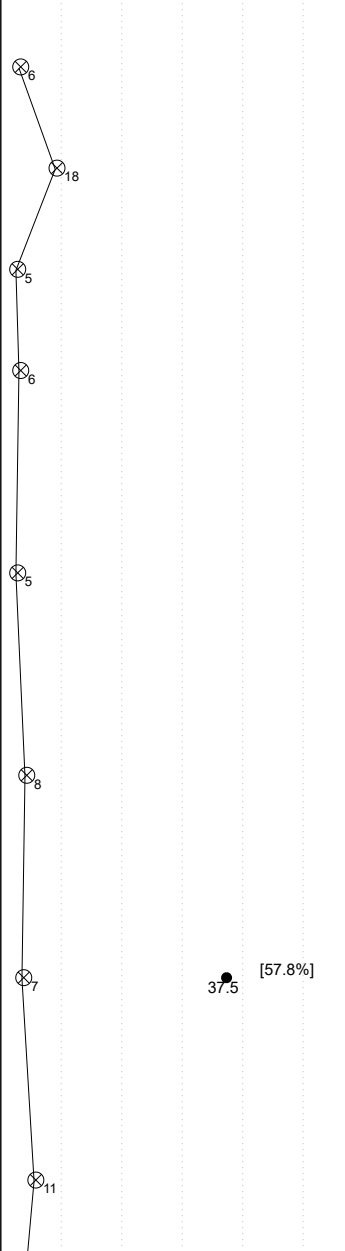
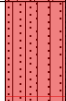
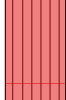
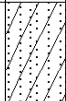
CLIENT: RaceTrac Petroleum, Inc.				PROJECT NO.: 10:10706-B		BORING NO.: B-25		SHEET: 1 of 1		
PROJECT NAME: RaceTrac Travel Center - Anvil Block Additional Borings				DRILLER/CONTRACTOR: Geo Lab, Inc. dba Geo Lab Drilling						
SITE LOCATION: 4443 Dunn Court, Ellenwood, Georgia 30294										
NORTHING:		EASTING:		STATION:		SURFACE ELEVATION: 893 +/-		LOSS OF CIRCULATION 		
								BOTTOM OF CASING 		
DEPTH (FT)	SAMPLE NUMBER	SAMPLE TYPE	SAMPLE DIST. (IN)	RECOVERY (IN)	DESCRIPTION OF MATERIAL	WATER LEVELS	ELEVATION (FT)	BLOWS/6"	Plastic Limit Water Content Liquid Limit X ————— ● ————— △	
									⊗ STANDARD PENETRATION BLOWS/FT	
									ROCK QUALITY DESIGNATION & RECOVERY	
									— RQD — REC	
									○ CALIBRATED PENETROMETER TON/SF [FINES CONTENT] %	
5	S-1	SS	18	18	Topsoil Thickness[2"] (CL FILL) FILL, SANDY LEAN CLAY, reddish brown to grayish brown, moist, firm			5-3-4 (7)	⊗ ₇	
	S-2	SS	18	18				3-3-3 (6)	⊗ ₆	
	S-3	SS	18	18	(SC FILL) FILL, CLAYEY SAND, tannish brown to orangish brown, moist, loose		888	3-3-7 (10)	⊗ ₁₀	
10	S-4	SS	18	18	(SM FILL) FILL, SILTY SAND, contains slight mica, reddish brown to tannish brown, moist, loose to medium dense		883	4-4-5 (9)	⊗ ₉	
15	S-5	SS	18	18			878	5-8-6 (14)	⊗ ₁₄	
					END OF DRILLING AT 15 FT					
20							873			
25							868			
30							863			
THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY LINES BETWEEN SOIL TYPES. IN-SITU THE TRANSITION MAY BE GRADUAL										
☒ WL (First Encountered) Dry					BORING STARTED: Dec 10 2020		CAVE IN DEPTH: 9			
▼ WL (Completion)					BORING COMPLETED: Dec 10 2020		HAMMER TYPE: Auto			
☒ WL (Seasonal High Water)					EQUIPMENT: ATV		LOGGED BY: NLC		DRILLING METHOD: HSA	
☒ WL (Stabilized)										
GEOTECHNICAL BOREHOLE LOG										

CLIENT: RaceTrac Petroleum, Inc.				PROJECT NO.: 10:10706-C		BORING NO.: B-26		SHEET: 1 of 2																																																																																																																																																																																																																																																																																																
PROJECT NAME: RaceTrac Travel Center - Anvil Block Deep Borings				DRILLER/CONTRACTOR: Geo Lab, Inc. dba Geo Lab Drilling																																																																																																																																																																																																																																																																																																				
SITE LOCATION: 4443 Dunn Court, Ellenwood, Georgia 30294								LOSS OF CIRCULATION 																																																																																																																																																																																																																																																																																																
NORTHING:		EASTING:		STATION:		SURFACE ELEVATION: 883 +/-		BOTTOM OF CASING 																																																																																																																																																																																																																																																																																																
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CLIENT: RaceTrac Petroleum, Inc.				PROJECT NO.: 10:10706-C		BORING NO.: B-26		SHEET: 2 of 2		
PROJECT NAME: RaceTrac Travel Center - Anvil Block Deep Borings				DRILLER/CONTRACTOR: Geo Lab, Inc.						
SITE LOCATION: 4443 Dunn Court, Ellenwood, Georgia 30294										
NORTHING:		EASTING:		STATION:		SURFACE ELEVATION: 883 +/-		LOSS OF CIRCULATION 		
								BOTTOM OF CASING 		
DEPTH (FT)	SAMPLE NUMBER	SAMPLE TYPE	SAMPLE DIST. (IN)	RECOVERY (IN)	DESCRIPTION OF MATERIAL	WATER LEVELS	ELEVATION (FT)	BLOWS/6"	Plastic Limit Water Content Liquid Limit X ————— ● ————— △	
									⊗ STANDARD PENETRATION BLOWS/FT ROCK QUALITY DESIGNATION & RECOVERY	
									— RQD — REC	
									○ CALIBRATED PENETROMETER TON/SF [FINES CONTENT] %	
35	S-9	SS	18	18	(ML) SANDY SILT, brown, moist, soft to firm (SM) SILTY SAND, contains mica, brown, moist, medium dense		848	3-8-8 (16)	⊗ ₁₆	
40	S-10	SS	18	18			843	3-6-7 (13)	⊗ ₁₃	
45	S-11	SS	18	18			838	9-11-16 (27)	⊗ ₂₇	
50	S-12	SS	15	15	(PWR)PARTIALLY WEATHERED ROCK SAMPLED AS SILTY SAND, gray, moist		833	15-17-50/3" (67/9")	⊗ _{67/9"}	
					END OF DRILLING AT 50 FT					
55							828			
60							823			
THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY LINES BETWEEN SOIL TYPES. IN-SITU THE TRANSITION MAY BE GRADUAL										
WL (First Encountered) 24					BORING STARTED: Dec 16 2020		CAVE IN DEPTH:			
WL (Completion)					BORING COMPLETED: Dec 16 2020		HAMMER TYPE: Auto			
WL (Seasonal High Water)					EQUIPMENT: ATV		LOGGED BY: FM1		DRILLING METHOD: HSA	
WL (Stabilized)										
GEOTECHNICAL BOREHOLE LOG										

CLIENT: RaceTrac Petroleum, Inc.				PROJECT NO.: 10:10706-C		BORING NO.: B-27		SHEET: 1 of 1		
PROJECT NAME: RaceTrac Travel Center - Anvil Block Deep Borings				DRILLER/CONTRACTOR: Geo Lab, Inc.						
SITE LOCATION: 4443 Dunn Court, Ellenwood, Georgia 30294										
NORTHING:		EASTING:		STATION:		SURFACE ELEVATION: 887 +/-		LOSS OF CIRCULATION 		
								BOTTOM OF CASING 		
DEPTH (FT)	SAMPLE NUMBER	SAMPLE TYPE	SAMPLE DIST. (IN)	RECOVERY (IN)	DESCRIPTION OF MATERIAL	WATER LEVELS	ELEVATION (FT)	BLOWS/6"	Plastic Limit Water Content Liquid Limit X ————— ● ————— △ ⊗ STANDARD PENETRATION BLOWS/FT ROCK QUALITY DESIGNATION & RECOVERY — RQD — REC ○ CALIBRATED PENETROMETER TON/SF [FINES CONTENT] %	
	S-1	SS	18	18	Topsoil Thickness[6"] (SC FILL) FILL, CLAYEY SAND, contains roots, dark brown, moist, very loose to very dense			2-8-7 (15)	⊗ ₁₅	
5	S-2	SS	18	18				2-2-2 (4)	⊗ ₄	
	S-3	SS	2	2				50/2" (50/2")	⊗ _{50/2"}	
10					Refusal encountered at 10 feet. END OF DRILLING AT 10 FT OFFSET BORING REFUSAL AT 11 FT		882			
15							877			
20							872			
25							867			
30							862			
							857			
THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY LINES BETWEEN SOIL TYPES. IN-SITU THE TRANSITION MAY BE GRADUAL										
☒ WL (First Encountered) Dry				BORING STARTED: Dec 16 2020		CAVE IN DEPTH:				
☒ WL (Completion)				BORING COMPLETED: Dec 16 2020		HAMMER TYPE: Auto				
☒ WL (Seasonal High Water)				EQUIPMENT: ATV		LOGGED BY: FM1		DRILLING METHOD: HSA		
☒ WL (Stabilized)										
GEOTECHNICAL BOREHOLE LOG										

CLIENT: RaceTrac Petroleum, Inc.				PROJECT NO.: 10:10706-C		BORING NO.: B-28		SHEET: 1 of 1		
PROJECT NAME: RaceTrac Travel Center - Anvil Block Deep Borings				DRILLER/CONTRACTOR: Geo Lab, Inc.						
SITE LOCATION: 4443 Dunn Court, Ellenwood, Georgia 30294										
NORTHING:		EASTING:		STATION:		SURFACE ELEVATION: 888 +/-		LOSS OF CIRCULATION 		
								BOTTOM OF CASING 		
DEPTH (FT)	SAMPLE NUMBER	SAMPLE TYPE	SAMPLE DIST. (IN)	RECOVERY (IN)	DESCRIPTION OF MATERIAL	WATER LEVELS	ELEVATION (FT)	BLOWS/6"	Plastic Limit Water Content Liquid Limit X ————— ● ————— △	
									⊗ STANDARD PENETRATION BLOWS/FT ROCK QUALITY DESIGNATION & RECOVERY	
									— RQD — REC	
									○ CALIBRATED PENETROMETER TON/SF [FINES CONTENT] %	
5	S-1	SS	18	18	Topsoil Thickness[6"] (ML FILL) FILL, SANDY SILT, dark brown to dark gray, moist, stiff		883	3-3-6 (9)	⊗ ₉	
	S-2	SS	18	18	(SM FILL) FILL, SILTY SAND, dark brown, moist, medium dense		883	4-3-12 (15)	⊗ ₁₅	
10					Refusal encountered at 6 feet. END OF DRILLING AT 6 FT OFFSET BORING REFUSAL AT 6 FT		878			
15							873			
20							868			
25							863			
30							858			
THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY LINES BETWEEN SOIL TYPES. IN-SITU THE TRANSITION MAY BE GRADUAL										
WL (First Encountered) Dry				BORING STARTED: Dec 16 2020		CAVE IN DEPTH:				
WL (Completion)				BORING COMPLETED: Dec 16 2020		HAMMER TYPE: Auto				
WL (Seasonal High Water)				EQUIPMENT: ATV		LOGGED BY: FM1		DRILLING METHOD: HSA		
WL (Stabilized)										
GEOTECHNICAL BOREHOLE LOG										

CLIENT: RaceTrac Petroleum, Inc.				PROJECT NO.: 10:10706-C		BORING NO.: B-29		SHEET: 1 of 2																																																																																																						
PROJECT NAME: RaceTrac Travel Center - Anvil Block Deep Borings				DRILLER/CONTRACTOR: Geo Lab, Inc. dba Geo Lab Drilling																																																																																																										
SITE LOCATION: 4443 Dunn Court, Ellenwood, Georgia 30294								LOSS OF CIRCULATION 																																																																																																						
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DEPTH (FT)	SAMPLE NUMBER	SAMPLE TYPE	SAMPLE DIST. (IN)	RECOVERY (IN)	DESCRIPTION OF MATERIAL	WATER LEVELS	ELEVATION (FT)	BLOWS/6"	Plastic Limit Water Content Liquid Limit X ● Δ																																																																																																					
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5	S-1	SS	18	18	Topsoil Thickness[6"] (SC FILL) FILL, CLAYEY SAND, contains roots, dark brown, moist, loose		884	3-3-3 (6)	⊗ ₆																																																																																																					
	S-2	SS	18	18	(SM FILL) FILL, SILTY SAND, contains wood, dark brown to gray, moist to wet, medium dense			4-7-11 (18)	⊗ ₁₈																																																																																																					
	S-3	SS	18	18	(ML FILL) FILL, SANDY SILT, contains wood, dark brown to dark gray, moist, firm			2-3-2 (5)	⊗ ₅																																																																																																					
	S-4	SS	18	18	(NO RECOVERY)			3-3-3 (6)	⊗ ₆																																																																																																					
15	S-5	SS	18	18	(SC) CLAYEY SAND, gray to tan, wet, loose		874	2-3-2 (5)	⊗ ₅																																																																																																					
	S-6	SS	18	18	(ML) SANDY SILT, gray to tan, moist, firm to stiff			2-4-4 (8)	⊗ ₈																																																																																																					
25	S-7	SS	18	18			864	2-2-5 (7)	⊗ ₇	37.5 [57.8%]																																																																																																				
	S-8	SS	18	18				6-6-5 (11)	⊗ ₁₁																																																																																																					
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WL (First Encountered) 27				BORING STARTED: Dec 16 2020				CAVE IN DEPTH:																																																																																																						
WL (Completion)				BORING COMPLETED: Dec 16 2020				HAMMER TYPE: Auto																																																																																																						
WL (Seasonal High Water)				EQUIPMENT: ATV				LOGGED BY: FM1																																																																																																						
WL (Stabilized)								DRILLING METHOD: HSA																																																																																																						
GEOTECHNICAL BOREHOLE LOG																																																																																																														

CLIENT: RaceTrac Petroleum, Inc.				PROJECT NO.: 10:10706-C		BORING NO.: B-29		SHEET: 2 of 2																																																																																																														
PROJECT NAME: RaceTrac Travel Center - Anvil Block Deep Borings				DRILLER/CONTRACTOR: Geo Lab, Inc.																																																																																																																		
SITE LOCATION: 4443 Dunn Court, Ellenwood, Georgia 30294										LOSS OF CIRCULATION 																																																																																																												
NORTHING:			EASTING:		STATION:		SURFACE ELEVATION: 889 +/-		BOTTOM OF CASING 																																																																																																													
<table><thead><tr><th rowspan="4">DEPTH (FT)</th><th rowspan="4">SAMPLE NUMBER</th><th rowspan="4">SAMPLE TYPE</th><th rowspan="4">SAMPLE DIST. (IN)</th><th rowspan="4">RECOVERY (IN)</th><th rowspan="4">DESCRIPTION OF MATERIAL</th><th rowspan="4">WATER LEVELS</th><th rowspan="4">ELEVATION (FT)</th><th rowspan="4">BLOWS/6"</th><th colspan="2">Plastic Limit Water Content Liquid Limit X ● — △</th></tr><tr><th colspan="2">⊗ STANDARD PENETRATION BLOWS/FT</th></tr><tr><th colspan="2">ROCK QUALITY DESIGNATION & RECOVERY</th></tr><tr><th colspan="2">— RQD — REC</th></tr><tr><td colspan="10"></td><td colspan="2">○ CALIBRATED PENETROMETER TON/SF [FINES CONTENT] %</td></tr></thead><tbody><tr><td>35</td><td>S-9</td><td>SS</td><td>18</td><td>18</td><td>(ML) SANDY SILT, gray to tan, moist, firm to stiff</td><td></td><td>854</td><td>4-2-3 (5)</td><td>⊗₅</td><td></td></tr><tr><td>40</td><td>S-10</td><td>SS</td><td>18</td><td>18</td><td>(NO RECOVERY)</td><td></td><td>849</td><td>4-2-2 (4)</td><td>⊗₄</td><td></td></tr><tr><td>45</td><td>S-11</td><td>SS</td><td>16</td><td>16</td><td>(PWR) PARTIALLY WEATHERED ROCK SAMPLED AS SILTY SAND, contains mica, gray, moist</td><td></td><td>844</td><td>14-22-50/4" (72/10")</td><td></td><td>⊗_{72/10"}</td></tr><tr><td colspan="5"></td><td colspan="2">Refusal encountered at 46 feet END OF DRILLING AT 46 FT</td><td colspan="5"></td></tr><tr><td>50</td><td></td><td></td><td></td><td></td><td></td><td></td><td>839</td><td></td><td></td><td></td></tr><tr><td>55</td><td></td><td></td><td></td><td></td><td></td><td></td><td>834</td><td></td><td></td><td></td></tr><tr><td>60</td><td></td><td></td><td></td><td></td><td></td><td></td><td>829</td><td></td><td></td><td></td></tr></tbody></table>												DEPTH (FT)	SAMPLE NUMBER	SAMPLE TYPE	SAMPLE DIST. (IN)	RECOVERY (IN)	DESCRIPTION OF MATERIAL	WATER LEVELS	ELEVATION (FT)	BLOWS/6"	Plastic Limit Water Content Liquid Limit X ● — △		⊗ STANDARD PENETRATION BLOWS/FT		ROCK QUALITY DESIGNATION & RECOVERY		— RQD — REC												○ CALIBRATED PENETROMETER TON/SF [FINES CONTENT] %		35	S-9	SS	18	18	(ML) SANDY SILT, gray to tan, moist, firm to stiff		854	4-2-3 (5)	⊗ ₅		40	S-10	SS	18	18	(NO RECOVERY)		849	4-2-2 (4)	⊗ ₄		45	S-11	SS	16	16	(PWR) PARTIALLY WEATHERED ROCK SAMPLED AS SILTY SAND, contains mica, gray, moist		844	14-22-50/4" (72/10")		⊗ _{72/10"}						Refusal encountered at 46 feet END OF DRILLING AT 46 FT							50							839				55							834				60							829			
DEPTH (FT)	SAMPLE NUMBER	SAMPLE TYPE	SAMPLE DIST. (IN)	RECOVERY (IN)	DESCRIPTION OF MATERIAL	WATER LEVELS	ELEVATION (FT)	BLOWS/6"	Plastic Limit Water Content Liquid Limit X ● — △																																																																																																													
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45	S-11	SS	16	16	(PWR) PARTIALLY WEATHERED ROCK SAMPLED AS SILTY SAND, contains mica, gray, moist		844	14-22-50/4" (72/10")		⊗ _{72/10"}																																																																																																												
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☒ WL (Stabilized)																																																																																																																						
GEOTECHNICAL BOREHOLE LOG																																																																																																																						

CLIENT: RaceTrac Petroleum, Inc.	PROJECT NO.: 10:10706-C	SHEET: 1 of 1	
PROJECT NAME: RaceTrac Travel Center - Anvil Block	TEST PIT NO.: TP-11	SURFACE ELEVATION: 900 +/-	
SITE LOCATION: 4443 Dunn Court, Ellenwood, Georgia 30294		STATION:	
NORTHING:	EASTING:		

DEPTH (FT)	WATER LEVELS	ELEVATION (FT)	DESCRIPTION OF MATERIAL	EXCAVATION EFFORT	DCP	QP (TSF)	SAMPLE NUMBER	MOISTURE CONTENT (%)
			Topsoil Thickness[3"]					
			(ML FILL) FILL, SANDY SILT, contains roots, orangish-brown, moist				S-1	
			(SM) SILTY SAND, orangish- red, moist				S-2	
5		895						
			(SM) SILTY SAND, light red, moist				S-3	
10		890	END OF TEST PIT AT 10 FT					
15								

REMARKS:

THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDRY LINES BETWEEN SOIL TYPES. IN-SITU THE TRANSITION MAY BE GRADUAL					
EXCAVATION EFFORT: E - EASY M - MEDIUM D - DIFFICULT VD - VERY DIFFICULT					
☐ WL (First Encountered) ☒ Dry	☒ WL (Seasonal High)	CONTRACTOR:	OPERATOR:	MAKE/MODEL:	
☒ WL (Completion)		Long's Backhoe	TL	John Deere/310 SG	
ECS REP.:	DATE COMPLETED:	UNITS:	CAVE-IN-DEPTH:		
	Dec 17 2020	English			
TEST PIT LOG					

CLIENT: RaceTrac Petroleum, Inc.	PROJECT NO.: 10:10706-C	SHEET: 1 of 1	
PROJECT NAME: RaceTrac Travel Center - Anvil Block	TEST PIT NO.: TP-12	SURFACE ELEVATION: 899 +/-	
SITE LOCATION: 4443 Dunn Court, Ellenwood, Georgia 30294		STATION:	
NORTHING:		EASTING:	

DEPTH (FT)	WATER LEVELS	ELEVATION (FT)	DESCRIPTION OF MATERIAL	EXCAVATION EFFORT	DCP	QP (TSF)	SAMPLE NUMBER	MOISTURE CONTENT (%)
			Topsoil Thickness[4"] (ML FILL) FILL, SANDY SILT, red- orange, moist				S-1	
5		894	(SM) SILTY SAND, orangish- tan, moist				S-2	
10		889	END OF TEST PIT AT 10 FT					
15								

REMARKS:

THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDRY LINES BETWEEN SOIL TYPES. IN-SITU THE TRANSITION MAY BE GRADUAL			
EXCAVATION EFFORT: E - EASY M - MEDIUM D - DIFFICULT VD - VERY DIFFICULT			
☐ WL (First Encountered) Dry	☐ WL (Seasonal High)	CONTRACTOR:	OPERATOR:
☒ WL (Completion)		Long's Backhoe	TL
MAKE/MODEL:		John Deere/310 SG	
ECS REP.:	DATE COMPLETED:	UNITS:	CAVE-IN-DEPTH:
CBS	Dec 17 2020	English	
TEST PIT LOG			

CLIENT: RaceTrac Petroleum, Inc.	PROJECT NO.: 10:10706-C	SHEET: 1 of 1	
PROJECT NAME: RaceTrac Travel Center - Anvil Block	TEST PIT NO.: TP-13	SURFACE ELEVATION: 883 +/-	
SITE LOCATION: 4443 Dunn Court, Ellenwood, Georgia 30294		STATION:	
NORTHING:	EASTING:		

DEPTH (FT)	WATER LEVELS	ELEVATION (FT)	DESCRIPTION OF MATERIAL	EXCAVATION EFFORT	DCP	QP (TSF)	SAMPLE NUMBER	MOISTURE CONTENT (%)
			Topsoil Thickness[5"]					
			(SC FILL) FILL, CLAYEY SAND, red- orange, moist					
			(ML FILL) FILL, SANDY SILT, contains mica, gray- tan, moist				S-1	
5		878	(SC FILL) FILL, CLAYEY SAND, contains mica, orange- brown, moist				S-2	
10		873	(SC FILL) FILL, CLAYEY SAND, contains mica, dark brown- purple, wet	▽				
			END OF TEST PIT AT 14 FT					
15								

REMARKS:

THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDRY LINES BETWEEN SOIL TYPES. IN-SITU THE TRANSITION MAY BE GRADUAL					
EXCAVATION EFFORT: E - EASY M - MEDIUM D - DIFFICULT VD - VERY DIFFICULT					
▽ WL (First Encountered) 10'	▽ WL (Seasonal High)	CONTRACTOR:	OPERATOR:	MAKE/MODEL:	
▼ WL (Completion)		Long's Backhoe	TL	John Deere/310 SG	
ECS REP.: CBS	DATE COMPLETED: Dec 17 2020	UNITS: English	CAVE-IN-DEPTH:		
TEST PIT LOG					

CLIENT: RaceTrac Petroleum, Inc.		PROJECT NO.: 10:10706-C	SHEET: 1 of 1
PROJECT NAME: RaceTrac Travel Center - Anvil Block		TEST PIT NO.: TP-14	SURFACE ELEVATION: 880 +/-
SITE LOCATION: 4443 Dunn Court, Ellenwood, Georgia 30294			STATION:
NORTHING:		EASTING:	



DEPTH (FT)	WATER LEVELS	ELEVATION (FT)	DESCRIPTION OF MATERIAL	EXCAVATION EFFORT	DCP	QP (TSF)	SAMPLE NUMBER	MOISTURE CONTENT (%)
			Topsoil Thickness[5"]					
			(SC FILL) FILL, CLAYEY SAND, contains mica, gray- tan, moist				S-1	
5		875	(SC FILL) FILL, CLAYEY SAND, contains mica, orange- dark brown, moist	▽			S-2	
10		870	(SC FILL) FILL, CLAYEY SAND, contains mica, dark brown- purple, wet				S-3	
			END OF TEST PIT AT 14 FT					
15								

REMARKS:

THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDRY LINES BETWEEN SOIL TYPES. IN-SITU THE TRANSITION MAY BE GRADUAL					
EXCAVATION EFFORT: E - EASY M - MEDIUM D - DIFFICULT VD - VERY DIFFICULT					
▽ WL (First Encountered)	6'	▽ WL (Seasonal High)	CONTRACTOR:	OPERATOR:	MAKE/MODEL:
▼ WL (Completion)			Long's Backhoe	TL	John Deere/310 SG
ECS REP.: CBS	DATE COMPLETED: Dec 17 2020	UNITS: English	CAVE-IN-DEPTH:		
TEST PIT LOG					

CLIENT: RaceTrac Petroleum, Inc.		PROJECT NO.: 10:10706-C	SHEET: 1 of 1
PROJECT NAME: RaceTrac Travel Center - Anvil Block		TEST PIT NO.: TP-15	SURFACE ELEVATION: 893 +/-
SITE LOCATION: 4443 Dunn Court, Ellenwood, Georgia 30294			STATION:
NORTHING:		EASTING:	



DEPTH (FT)	WATER LEVELS	ELEVATION (FT)	DESCRIPTION OF MATERIAL	EXCAVATION EFFORT	DCP	QP (TSF)	SAMPLE NUMBER	MOISTURE CONTENT (%)
			Topsoil Thickness[3"] (CL FILL) FILL, SANDY LEAN CLAY, contains mica, brown-red, moist				S-1	
5		888	(SC) CLAYEY SAND, orangish- red, moist				S-2	
10		883	(SP/SW) POORLY GRADED/WELL-GRADED SAND, light brown, wet	▽			S-3	
15			END OF TEST PIT AT 14 FT					

REMARKS:

THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDRY LINES BETWEEN SOIL TYPES. IN-SITU THE TRANSITION MAY BE GRADUAL					
EXCAVATION EFFORT: E - EASY M - MEDIUM D - DIFFICULT VD - VERY DIFFICULT					
▽ WL (First Encountered)	8'	▽ WL (Seasonal High)	CONTRACTOR:	OPERATOR:	MAKE/MODEL:
▼ WL (Completion)			Long's Backhoe	TL	John Deere/310 SG
ECS REP.: CBS	DATE COMPLETED: Dec 17 2020	UNITS: English	CAVE-IN-DEPTH:		
TEST PIT LOG					



ECS Southeast, LLP
1281 Kennestone Circle
Suite 200
Marietta, GA 30066
Phone: 770-590-1971
Fax: 770-590-1975

TEST PIT PHOTO LOG

Project Name: RaceTrac Travel Center – Anvil Block

Project Number: 10:10706-C

Project Location: Ellenwood, Georgia

Date: 12/17/20



Photograph 1: Test Pit TP-11



Photograph 2: Excavated materials from Test Pit TP-11



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Fax: 770-590-1975

TEST PIT PHOTO LOG

Project Name: RaceTrac Travel Center – Anvil Block

Project Number: 10:10706-C

Project Location: Ellenwood, Georgia

Date: 12/17/20



Photograph 3: Test Pit TP-12



Photograph 4: Excavated materials from Test Pit TP-12.



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Marietta, GA 30066
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TEST PIT PHOTO LOG

Project Name: RaceTrac Travel Center – Anvil Block

Project Number: 10:10706-C

Project Location: Ellenwood, Georgia

Date: 12/17/20



Photograph 5: Test Pit TP-13



Photograph 6: Excavated materials from Test Pit TP-13



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TEST PIT PHOTO LOG

Project Name: RaceTrac Travel Center – Anvil Block

Project Number: 10:10706-C

Project Location: Ellenwood, Georgia

Date: 12/17/20



Photograph 7: Test Pit TP-14



Photograph 8: Excavated materials from Test Pit TP-14



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TEST PIT PHOTO LOG

Project Name: RaceTrac Travel Center – Anvil Block

Project Number: 10:10706-C

Project Location: Ellenwood, Georgia

Date: 12/17/20



Photograph 9: Test Pit TP-15



Photograph 10: Excavated materials from Test Pit TP-5

CLIENT: RaceTrac Petroleum, Inc.		PROJECT NO.: 10:10706-B	SHEET: 1 of 1	
PROJECT NAME: RaceTrac Travel Center - Anvil Block Additional Borings		HAND AUGER NO.: P-1	SURFACE ELEVATION: 901 +/-	
SITE LOCATION: 4443 Dunn Court, Ellenwood, Georgia 30294			STATION:	
NORTHING:		EASTING:		

DEPTH (FT)	WATER LEVELS	ELEVATION (FT)	DESCRIPTION OF MATERIAL	EXCAVATION EFFORT	DCP	SAMPLE NUMBER	FINES CONTENT (%)	MOISTURE CONTENT (%)
			Topsoil Thickness[2"] (ML FILL) FILL, SANDY SILT, contains roots, red, moist			S-1		
			(ML FILL) FILL, SANDY SILT, contains mica, light brown to red, moist					
			(ML FILL) FILL, SANDY SILT, contains mica, orangish red, moist	E				
5		896	END OF DRILLING AT 5 FT					
10		891						
15								

REMARKS:							
THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY LINES BETWEEN SOIL TYPES. IN-SITU THE TRANSITION MAY BE GRADUAL							
EXCAVATION EFFORT: E - EASY M - MEDIUM D - DIFFICULT VD - VERY DIFFICULT							
☐ WL (First Encountered) Dry		☒ WL (Seasonal High)		ECS REP:	DATE COMPLETED:	UNITS:	CAVE-IN-DEPTH:
☒ WL (Completion)				CBS	Dec 11 2020	English	
HAND AUGER LOG							

CLIENT: RaceTrac Petroleum, Inc.	PROJECT NO.: 10:10706-B	SHEET: 1 of 1	
PROJECT NAME: RaceTrac Travel Center - Anvil Block Additional Borings	HAND AUGER NO.: P-2	SURFACE ELEVATION: 899 +/-	
SITE LOCATION: 4443 Dunn Court, Ellenwood, Georgia 30294		STATION:	
NORTHING:		EASTING:	

DEPTH (FT)	WATER LEVELS	ELEVATION (FT)	DESCRIPTION OF MATERIAL	EXCAVATION EFFORT	DCP	SAMPLE NUMBER	FINES CONTENT (%)	MOISTURE CONTENT (%)
			Topsoil Thickness[2"] (ML FILL) FILL, SANDY SILT, contains mica and roots, reddish brown, moist			S-1		
			(ML/CL) SANDY CLAYEY SILT, contains mica, light brown to reddish brown, moist			S-2	69	22.5
						S-3		
5		894	END OF DRILLING AT 5 FT					
10		889						
15								

REMARKS:							
THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY LINES BETWEEN SOIL TYPES. IN-SITU THE TRANSITION MAY BE GRADUAL							
EXCAVATION EFFORT: E - EASY M - MEDIUM D - DIFFICULT VD - VERY DIFFICULT							
☐ WL (First Encountered) Dry	☒ WL (Seasonal High)	ECS REP:	DATE COMPLETED:	UNITS:	CAVE-IN-DEPTH:		
☒ WL (Completion)		CBS	Dec 11 2020	English			
HAND AUGER LOG							

CLIENT: RaceTrac Petroleum, Inc.	PROJECT NO.: 10:10706-B	SHEET: 1 of 1	
PROJECT NAME: RaceTrac Travel Center - Anvil Block Additional Borings	HAND AUGER NO.: P-3	SURFACE ELEVATION: 893 +/-	
SITE LOCATION: 4443 Dunn Court, Ellenwood, Georgia 30294		STATION:	
NORTHING:		EASTING:	

DEPTH (FT)	WATER LEVELS	ELEVATION (FT)	DESCRIPTION OF MATERIAL	EXCAVATION EFFORT	DCP	SAMPLE NUMBER	FINES CONTENT (%)	MOISTURE CONTENT (%)
			Topsoil Thickness[3"]			S-1		
			(ML FILL) FILL, SANDY SILT, contains mica and roots, reddish brown, moist					
			(ML FILL) FILL, SANDY SILT, reddish brown to orangish red			S-2		
				E		S-3	65	20.2
5		888	END OF DRILLING AT 5 FT					
10		883						
15								

REMARKS:							
THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY LINES BETWEEN SOIL TYPES. IN-SITU THE TRANSITION MAY BE GRADUAL							
EXCAVATION EFFORT: E - EASY M - MEDIUM D - DIFFICULT VD - VERY DIFFICULT							
☐ WL (First Encountered) Dry	☒ WL (Seasonal High)	ECS REP:	DATE COMPLETED:	UNITS:	CAVE-IN-DEPTH:		
☒ WL (Completion)		CBS	Dec 11 2020	English			
HAND AUGER LOG							

WILDCAT DYNAMIC CONE LOG

Page 1 of 1

ECS Southeast, LLP
1281 Kennestone Circle, Suite 200
Marietta, GA 30066

PROJECT NUMBER: 10:10796
DATE STARTED: 12-11-2020
DATE COMPLETED: 12-11-2020

HOLE #: WDCP-1
CREW: CBS
PROJECT: RaceTrac Travel Center - Anvil Block
ADDRESS: 4443 Old Dunn Court
LOCATION: Ellenwood, GA

SURFACE ELEVATION: 901 +/-
WATER ON COMPLETION:
HAMMER WEIGHT: 35 lbs.
CONE AREA: 10 sq. cm

DEPTH	BLOWS PER 10 cm	RESISTANCE Kg/cm ²	GRAPH OF CONE RESISTANCE 0 50 100 150	N'	TESTED CONSISTENCY	
					NON-COHESIVE	COHESIVE
-	1	4.4	•	1	VERY LOOSE	VERY SOFT
-	1	4.4	•	1	VERY LOOSE	VERY SOFT
- 1 ft	1	4.4	•	1	VERY LOOSE	VERY SOFT
-	2	8.9	••	2	VERY LOOSE	SOFT
-	11	48.8	••••••••••	13	MEDIUM DENSE	STIFF
- 2 ft	4	17.8	••••	5	LOOSE	MEDIUM STIFF
-	2	8.9	••	2	VERY LOOSE	SOFT
-	2	8.9	••	2	VERY LOOSE	SOFT
- 3 ft	2	8.9	••	2	VERY LOOSE	SOFT
- 1 m	3	13.3	•••	3	VERY LOOSE	SOFT
-	3	11.6	•••	3	VERY LOOSE	SOFT
- 4 ft	3	11.6	•••	3	VERY LOOSE	SOFT
-	6	23.2	•••••	6	LOOSE	MEDIUM STIFF
-	5	19.3	••••	5	LOOSE	MEDIUM STIFF
- 5 ft	6	23.2	•••••	6	LOOSE	MEDIUM STIFF
-	5	19.3	••••	5	LOOSE	MEDIUM STIFF
-	6	23.2	•••••	6	LOOSE	MEDIUM STIFF
- 6 ft	5	19.3	••••	5	LOOSE	MEDIUM STIFF
-	7	27.0	•••••	7	LOOSE	MEDIUM STIFF
- 2 m	9	34.7	•••••••	9	LOOSE	STIFF
- 7 ft	17	58.1	••••••••••	16	MEDIUM DENSE	VERY STIFF
-	17	58.1	••••••••••	16	MEDIUM DENSE	VERY STIFF
-	19	65.0	••••••••••	18	MEDIUM DENSE	VERY STIFF
- 8 ft	20	68.4	••••••••••	19	MEDIUM DENSE	VERY STIFF
-	24	82.1	••••••••••••	23	MEDIUM DENSE	VERY STIFF
-	26	88.9	•••••••••••••	25	MEDIUM DENSE	VERY STIFF
- 9 ft	23	78.7	••••••••••••	22	MEDIUM DENSE	VERY STIFF
-	20	68.4	••••••••••	19	MEDIUM DENSE	VERY STIFF
-	34	116.3	••••••••••••••••	25+	DENSE	HARD
- 3 m 10 ft	30	102.6	••••••••••••••	25+	MEDIUM DENSE	VERY STIFF
-	30	91.8	••••••~	25+	MEDIUM DENSE	VERY STIFF
-	32	97.9	••••••~	25+	MEDIUM DENSE	VERY STIFF
-			Equipment Refusal @ 10'-8"			
- 11 ft						
-						
- 12 ft						
-						
- 4 m 13 ft						

WILDCAT DYNAMIC CONE LOG

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ECS Southeast, LLP
1281 Kennestone Circle, Suite 200
Marietta, GA 30066

PROJECT NUMBER: 10:10796
DATE STARTED: 12-11-2020
DATE COMPLETED: 12-11-2020

HOLE #: WDCP-2
CREW: CBS
PROJECT: RaceTrac Travel Center - Anvil Block
ADDRESS: 4443 Old Dunn Court
LOCATION: Ellenwood, GA

SURFACE ELEVATION: 899 +/-
WATER ON COMPLETION:
HAMMER WEIGHT: 35 lbs.
CONE AREA: 10 sq. cm

DEPTH	BLOWS PER 10 cm	RESISTANCE Kg/cm ²	GRAPH OF CONE RESISTANCE 0 50 100 150	N'	TESTED CONSISTENCY	
					NON-COHESIVE	COHESIVE
-	1	4.4	•	1	VERY LOOSE	VERY SOFT
-	2	8.9	••	2	VERY LOOSE	SOFT
- 1 ft	3	13.3	•••	3	VERY LOOSE	SOFT
-	3	13.3	•••	3	VERY LOOSE	SOFT
-	7	31.1	•••••••	8	LOOSE	MEDIUM STIFF
- 2 ft	3	13.3	•••	3	VERY LOOSE	SOFT
-	3	13.3	•••	3	VERY LOOSE	SOFT
-	4	17.8	••••	5	LOOSE	MEDIUM STIFF
- 3 ft	6	26.6	•••••	7	LOOSE	MEDIUM STIFF
- 1 m	7	31.1	•••••••	8	LOOSE	MEDIUM STIFF
-	4	15.4	••••	4	VERY LOOSE	SOFT
- 4 ft	6	23.2	•••••	6	LOOSE	MEDIUM STIFF
-	10	38.6	••••••••	11	MEDIUM DENSE	STIFF
-	15	57.9	••••••••••	16	MEDIUM DENSE	VERY STIFF
- 5 ft	14	54.0	•••••••••	15	MEDIUM DENSE	STIFF
-	10	38.6	•••••••	11	MEDIUM DENSE	STIFF
-	11	42.5	••••••••	12	MEDIUM DENSE	STIFF
- 6 ft	9	34.7	•••••••	9	LOOSE	STIFF
-	8	30.9	••••••	8	LOOSE	MEDIUM STIFF
- 2 m	5	19.3	••••	5	LOOSE	MEDIUM STIFF
- 7 ft	7	23.9	•••••	6	LOOSE	MEDIUM STIFF
-	11	37.6	•••••••	10	LOOSE	STIFF
-	9	30.8	••••••	8	LOOSE	MEDIUM STIFF
- 8 ft	24	82.1	••••••••••••••	23	MEDIUM DENSE	VERY STIFF
-	31	106.0	••••••••••••••••	25+	MEDIUM DENSE	VERY STIFF
-	35	119.7	••••••••••••••••••	25+	DENSE	HARD
- 9 ft	47	160.7	••••••••••••••••••••	25+	DENSE	HARD
-	36	123.1	••••••••••••••••••	25+	DENSE	HARD
- 3 m	10 ft		Equipment Refusal @ 9'-2"			
-						
-						
-	11 ft					
-						
-	12 ft					
-						
- 4 m	13 ft					

WILDCAT DYNAMIC CONE LOG

Page 1 of 2

ECS Southeast, LLP
1281 Kennestone Circle, Suite 200
Marietta, GA 30066

PROJECT NUMBER: 10:10796
DATE STARTED: 12-11-2020
DATE COMPLETED: 12-11-2020

HOLE #: WDCP-3
CREW: FM
PROJECT: RaceTrac Travel Center - Anvil Block
ADDRESS: 4443 Old Dunn Court
LOCATION: Ellenwood, GA

SURFACE ELEVATION: 893 +/-
WATER ON COMPLETION:
HAMMER WEIGHT: 35 lbs.
CONE AREA: 10 sq. cm

DEPTH	BLOWS PER 10 cm	RESISTANCE Kg/cm ²	GRAPH OF CONE RESISTANCE				N'	TESTED CONSISTENCY	
			0	50	100	150		NON-COHESIVE	COHESIVE
-	2	8.9	••				2	VERY LOOSE	SOFT
-	2	8.9	••				2	VERY LOOSE	SOFT
- 1 ft	3	13.3	•••				3	VERY LOOSE	SOFT
-	3	13.3	•••				3	VERY LOOSE	SOFT
-	2	8.9	••				2	VERY LOOSE	SOFT
- 2 ft	3	13.3	•••				3	VERY LOOSE	SOFT
-	3	13.3	•••				3	VERY LOOSE	SOFT
-	4	17.8	•••••				5	LOOSE	MEDIUM STIFF
- 3 ft	3	13.3	•••				3	VERY LOOSE	SOFT
- 1 m	3	13.3	•••				3	VERY LOOSE	SOFT
-	3	11.6	•••				3	VERY LOOSE	SOFT
- 4 ft	3	11.6	•••				3	VERY LOOSE	SOFT
-	2	7.7	••				2	VERY LOOSE	SOFT
-	3	11.6	•••				3	VERY LOOSE	SOFT
- 5 ft	3	11.6	•••				3	VERY LOOSE	SOFT
-	2	7.7	••				2	VERY LOOSE	SOFT
-	2	7.7	••				2	VERY LOOSE	SOFT
- 6 ft	2	7.7	••				2	VERY LOOSE	SOFT
-	4	15.4	••••				4	VERY LOOSE	SOFT
- 2 m	4	15.4	••••				4	VERY LOOSE	SOFT
- 7 ft	7	23.9	•••••				6	LOOSE	MEDIUM STIFF
-	14	47.9	••••••••••				13	MEDIUM DENSE	STIFF
-	9	30.8	•••••••				8	LOOSE	MEDIUM STIFF
- 8 ft	6	20.5	•••••				5	LOOSE	MEDIUM STIFF
-	8	27.4	••••••				7	LOOSE	MEDIUM STIFF
-	9	30.8	•••••••				8	LOOSE	MEDIUM STIFF
- 9 ft	8	27.4	••••••				7	LOOSE	MEDIUM STIFF
-	8	27.4	••••••				7	LOOSE	MEDIUM STIFF
-	14	47.9	••••••••••				13	MEDIUM DENSE	STIFF
- 3 m 10 ft	17	58.1	••••••••••••••				16	MEDIUM DENSE	VERY STIFF
-	13	39.8	••••••••••				11	MEDIUM DENSE	STIFF
-	9	27.5	••••••				7	LOOSE	MEDIUM STIFF
-	5	15.3	••••				4	VERY LOOSE	SOFT
- 11 ft	5	15.3	••••				4	VERY LOOSE	SOFT
-	10	30.6	•••••••				8	LOOSE	MEDIUM STIFF
-	10	30.6	•••••••				8	LOOSE	MEDIUM STIFF
- 12 ft	17	52.0	••••••••••••••				14	MEDIUM DENSE	STIFF
-	12	36.7	••••••••				10	LOOSE	STIFF
-	16	49.0	••••••••••				13	MEDIUM DENSE	STIFF
- 4 m 13 ft	11	33.7	•••••••				9	LOOSE	STIFF

	BLOWS	RESISTANCE	GRAPH OF CONE RESISTANCE	N'	TESTED CONSISTENCY	
DEPTH	PER 10 cm	Kg/cm²	0 50 100 150		NON-COHESIVE	COHESIVE
-	11				VERY DENSE	HARD
-	8	22.2		6	LOOSE	MEDIUM STIFF
-	14 ft	8	22.2	6	LOOSE	MEDIUM STIFF
-	7	19.4		5	LOOSE	MEDIUM STIFF
-	5	13.9		3	VERY LOOSE	SOFT
-	15 ft	2	.	1	VERY LOOSE	VERY SOFT
-	3	8.3	..	2	VERY LOOSE	SOFT
-	3	8.3	..	2	VERY LOOSE	SOFT
-	16 ft	3	..	2	VERY LOOSE	SOFT
-	5 m	5		3	VERY LOOSE	SOFT
-	6	15.2		4	VERY LOOSE	SOFT
-	17 ft	6		4	VERY LOOSE	SOFT
-	6	15.2		4	VERY LOOSE	SOFT
-	5	12.7	...	3	VERY LOOSE	SOFT
-	18 ft	7		5	LOOSE	MEDIUM STIFF
-	7	17.8		5	LOOSE	MEDIUM STIFF
-	12	30.5		8	LOOSE	MEDIUM STIFF
-	19 ft	9		6	LOOSE	MEDIUM STIFF
-	9	22.9		6	LOOSE	MEDIUM STIFF
-	6 m	8		5	LOOSE	MEDIUM STIFF
-	20 ft	8		5	LOOSE	MEDIUM STIFF
-	9	21.0		5	LOOSE	MEDIUM STIFF
-	12	28.0		7	LOOSE	MEDIUM STIFF
-	21 ft		WDCP Terminated @ 20'-8"			
-	22 ft					
-	23 ft					
-	24 ft					
-	25 ft					
-	26 ft					
-	8 m					
-	27 ft					
-	28 ft					
-	29 ft					
-	9 m					

Laboratory Testing Summary

Sample Source	Sample Number	Depth (feet)	^MC (%)	Soil Type	Atterberg Limits			**Percent Passing No. 200 Sieve	Moisture - Density		CBR (%)		#Organic Content (%)
					LL	PL	PI		Maximum Density (pcf)	Optimum Moisture (%)	0.1 in.	0.2 in.	
B-26	S-4	8.5-10	22.7	CH	52	25	27	63.3					
B-29	S-7	23.5-25	37.5	ML	NP	NP	NP	57.8					

Notes: See test reports for test method, ^ASTM D2216-19, *ASTM D2488, **ASTM D1140-17, #ASTM D2974-20e1

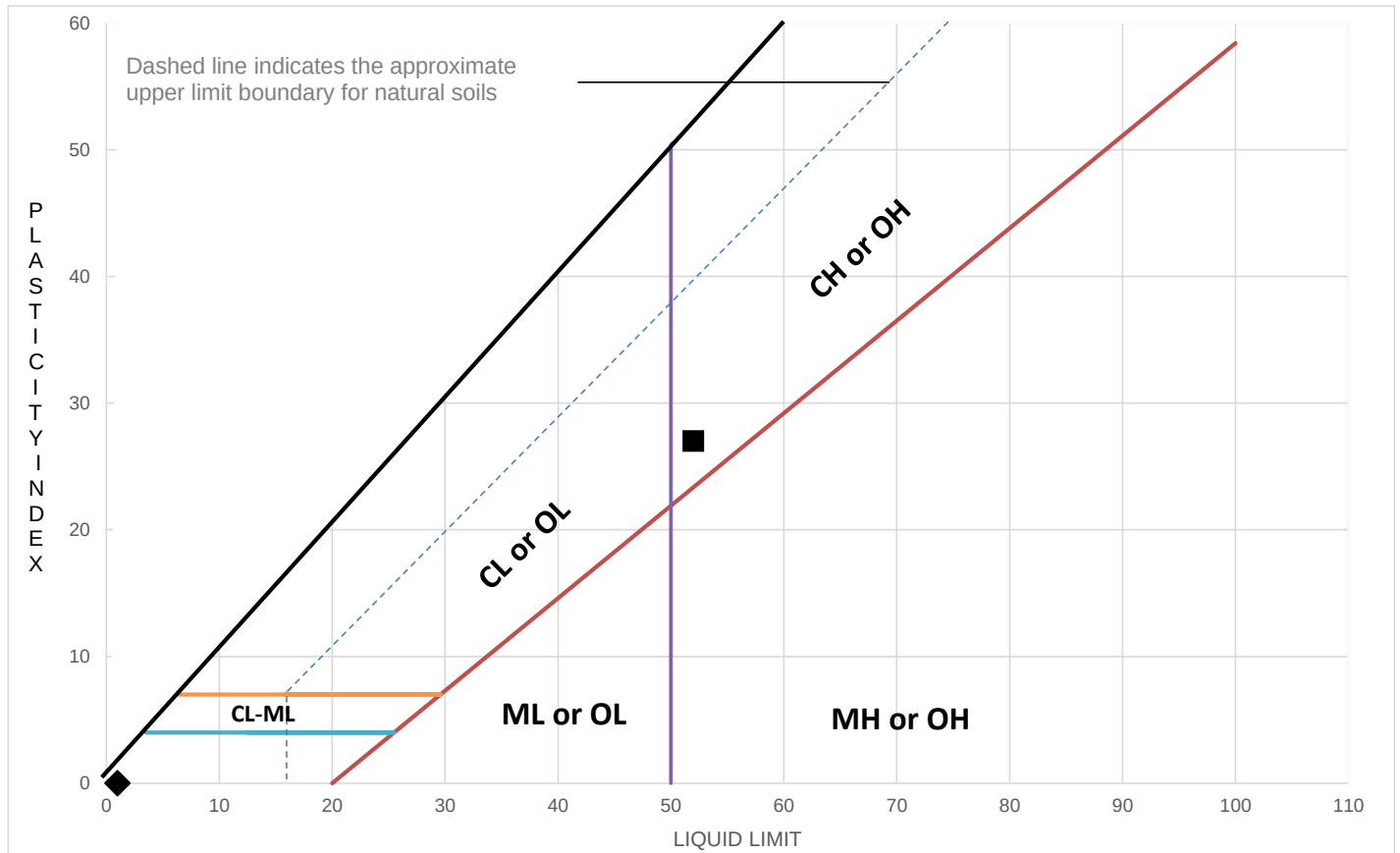
Definitions: MC: Moisture Content, Soil Type: USCS (Unified Soil Classification System), LL: Liquid Limit, PL: Plastic Limit, PI: Plasticity Index, CBR: California Bearing Ratio, OC: Organic Content

Project:	RaceTrac Travel Center - Anvil Block Deep Borings	Project No.:	10:10706-C
Client:	RaceTrac Petroleum, Inc.	Date Reported:	12/28/2020

Office / Lab	Address	Office Number / Fax
 ECS Southeast LLP - Marietta	1281 Kennestone Circle NE Suite 200 Marietta, GA 30066	(770)590-1971 (770)590-1975

Tested by	Checked by	Approved by	Date Received
KShah	KShah	KShah	

LIQUID AND PLASTIC LIMITS TEST REPORT



TEST RESULTS (ASTM D4318-10 (SINGLE POINT TEST))

	Sample Location	Sample Number	Sample Depth (ft)	LL	PL	PI	%<#40	%<#200	AASHTO	USCS	Material Description
■	B-26	S-4	8.5-10	52	25	27		63.3			
◆	B-29	S-7	23.5-25	NP	NP	NP		57.8			

Project: RaceTrac Travel Center - Anvil Block Deep Borings
Client: RaceTrac Petroleum, Inc.

Project No.: 10:10706-C
Date Reported: 12/28/2020



Office / Lab
ECS Southeast LLP - Marietta

Address
1281 Kennestone Circle NE
Suite 200
Marietta, GA 30066

Office Number / Fax
(770)590-1971
(770)590-1975

Tested by	Checked by	Approved by	Date Received
KShah	KShah	KShah	

Laboratory Testing Summary

Sample Source	Sample Number	Depth (feet)	^MC (%)	Soil Type	Atterberg Limits			**Percent Passing No. 200 Sieve	Moisture - Density		CBR (%)		#Organic Content (%)
					LL	PL	PI		Maximum Density (pcf)	Optimum Moisture (%)	0.1 in.	0.2 in.	
P-2	S-2	2	22.5	ML/CL FILL	42	26	16	68.7					
P-3	S-3	4	20.2	ML FILL	48	31	17	65					

Notes: See test reports for test method, ^ASTM D2216-19, *ASTM D2488, **ASTM D1140-17, #ASTM D2974-20e1

Definitions: MC: Moisture Content, Soil Type: USCS (Unified Soil Classification System), LL: Liquid Limit, PL: Plastic Limit, PI: Plasticity Index, CBR: California Bearing Ratio, OC: Organic Content

Project: RaceTrac Travel Center - Anvil Block Additional Borings
Client: RaceTrac Petroleum, Inc.

Project No.: 10:10706-B
Date Reported: 12/28/2020



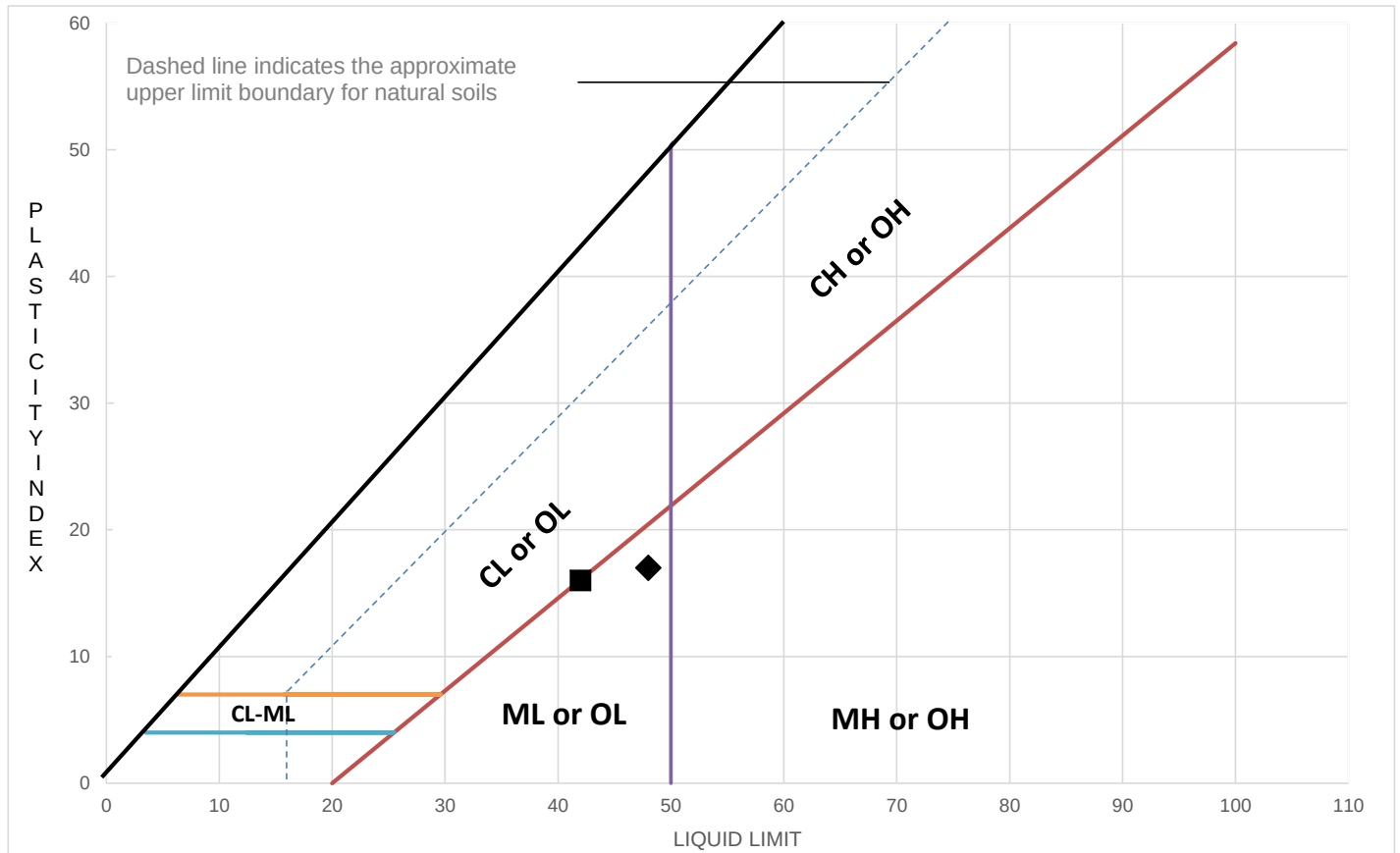
Office / Lab
ECS Southeast LLP - Marietta

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Marietta, GA 30066

Office Number / Fax
(770)590-1971
(770)590-1975

Tested by	Checked by	Approved by	Date Received
KShah	KShah	KShah	

LIQUID AND PLASTIC LIMITS TEST REPORT



TEST RESULTS (ASTM D4318-10 (SINGLE POINT TEST))

	Sample Location	Sample Number	Sample Depth (ft)	LL	PL	PI	%<#40	%<#200	AASHTO	USCS	Material Description
■	P-2	S-2	2	42	26	16		68.7			
◆	P-3	S-3	4	48	31	17		65.0			

Project: RaceTrac Travel Center - Anvil Block Additional Borings
Client: RaceTrac Petroleum, Inc.

Project No.: 10:10706-B
Date Reported: 12/28/2020



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1281 Kennestone Circle NE
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Tested by	Checked by	Approved by	Date Received
KShah	KShah	KShah	

Important Information about This Geotechnical-Engineering Report

Subsurface problems are a principal cause of construction delays, cost overruns, claims, and disputes.

While you cannot eliminate all such risks, you can manage them. The following information is provided to help.

The Geoprofessional Business Association (GBA) has prepared this advisory to help you – assumedly a client representative – interpret and apply this geotechnical-engineering report as effectively as possible. In that way, clients can benefit from a lowered exposure to the subsurface problems that, for decades, have been a principal cause of construction delays, cost overruns, claims, and disputes. If you have questions or want more information about any of the issues discussed below, contact your GBA-member geotechnical engineer. Active involvement in the Geoprofessional Business Association exposes geotechnical engineers to a wide array of risk-confrontation techniques that can be of genuine benefit for everyone involved with a construction project.

Geotechnical-Engineering Services Are Performed for Specific Purposes, Persons, and Projects

Geotechnical engineers structure their services to meet the specific needs of their clients. A geotechnical-engineering study conducted for a given civil engineer will not likely meet the needs of a civil-works constructor or even a different civil engineer. Because each geotechnical-engineering study is unique, each geotechnical-engineering report is unique, prepared *solely* for the client. *Those who rely on a geotechnical-engineering report prepared for a different client can be seriously misled.* No one except authorized client representatives should rely on this geotechnical-engineering report without first conferring with the geotechnical engineer who prepared it. *And no one – not even you – should apply this report for any purpose or project except the one originally contemplated.*

Read this Report in Full

Costly problems have occurred because those relying on a geotechnical-engineering report did not read it *in its entirety*. Do not rely on an executive summary. Do not read selected elements only. *Read this report in full.*

You Need to Inform Your Geotechnical Engineer about Change

Your geotechnical engineer considered unique, project-specific factors when designing the study behind this report and developing the confirmation-dependent recommendations the report conveys. A few typical factors include:

- the client's goals, objectives, budget, schedule, and risk-management preferences;
- the general nature of the structure involved, its size, configuration, and performance criteria;
- the structure's location and orientation on the site; and
- other planned or existing site improvements, such as retaining walls, access roads, parking lots, and underground utilities.

Typical changes that could erode the reliability of this report include those that affect:

- the site's size or shape;
- the function of the proposed structure, as when it's changed from a parking garage to an office building, or from a light-industrial plant to a refrigerated warehouse;
- the elevation, configuration, location, orientation, or weight of the proposed structure;
- the composition of the design team; or
- project ownership.

As a general rule, *always* inform your geotechnical engineer of project changes – even minor ones – and request an assessment of their impact. *The geotechnical engineer who prepared this report cannot accept responsibility or liability for problems that arise because the geotechnical engineer was not informed about developments the engineer otherwise would have considered.*

This Report May Not Be Reliable

Do not rely on this report if your geotechnical engineer prepared it:

- for a different client;
- for a different project;
- for a different site (that may or may not include all or a portion of the original site); or
- before important events occurred at the site or adjacent to it; e.g., man-made events like construction or environmental remediation, or natural events like floods, droughts, earthquakes, or groundwater fluctuations.

Note, too, that it could be unwise to rely on a geotechnical-engineering report whose reliability may have been affected by the passage of time, because of factors like changed subsurface conditions; new or modified codes, standards, or regulations; or new techniques or tools. *If your geotechnical engineer has not indicated an "apply-by" date on the report, ask what it should be, and, in general, if you are the least bit uncertain about the continued reliability of this report, contact your geotechnical engineer before applying it.* A minor amount of additional testing or analysis – if any is required at all – could prevent major problems.

Most of the "Findings" Related in This Report Are Professional Opinions

Before construction begins, geotechnical engineers explore a site's subsurface through various sampling and testing procedures. *Geotechnical engineers can observe actual subsurface conditions only at those specific locations where sampling and testing were performed.* The data derived from that sampling and testing were reviewed by your geotechnical engineer, who then applied professional judgment to form opinions about subsurface conditions throughout the site. Actual sitewide-subsurface conditions may differ – maybe significantly – from those indicated in this report. Confront that risk by retaining your geotechnical engineer to serve on the design team from project start to project finish, so the individual can provide informed guidance quickly, whenever needed.

This Report's Recommendations Are Confirmation-Dependent

The recommendations included in this report – including any options or alternatives – are confirmation-dependent. In other words, *they are not final*, because the geotechnical engineer who developed them relied heavily on judgment and opinion to do so. Your geotechnical engineer can finalize the recommendations *only after observing actual subsurface conditions* revealed during construction. If through observation your geotechnical engineer confirms that the conditions assumed to exist actually do exist, the recommendations can be relied upon, assuming no other changes have occurred. *The geotechnical engineer who prepared this report cannot assume responsibility or liability for confirmation-dependent recommendations if you fail to retain that engineer to perform construction observation.*

This Report Could Be Misinterpreted

Other design professionals' misinterpretation of geotechnical-engineering reports has resulted in costly problems. Confront that risk by having your geotechnical engineer serve as a full-time member of the design team, to:

- confer with other design-team members,
- help develop specifications,
- review pertinent elements of other design professionals' plans and specifications, and
- be on hand quickly whenever geotechnical-engineering guidance is needed.

You should also confront the risk of constructors misinterpreting this report. Do so by retaining your geotechnical engineer to participate in prebid and preconstruction conferences and to perform construction observation.

Give Constructors a Complete Report and Guidance

Some owners and design professionals mistakenly believe they can shift unanticipated-subsurface-conditions liability to constructors by limiting the information they provide for bid preparation. To help prevent the costly, contentious problems this practice has caused, include the complete geotechnical-engineering report, along with any attachments or appendices, with your contract documents, *but be certain to note conspicuously that you've included the material for informational purposes only*. To avoid misunderstanding, you may also want to note that "informational purposes" means constructors have no right to rely on the interpretations, opinions, conclusions, or recommendations in the report, but they may rely on the factual data relative to the specific times, locations, and depths/elevations referenced. Be certain that constructors know they may learn about specific project requirements, including options selected from the report, *only* from the design drawings and specifications. Remind constructors that they may

perform their own studies if they want to, and *be sure to allow enough time* to permit them to do so. Only then might you be in a position to give constructors the information available to you, while requiring them to at least share some of the financial responsibilities stemming from unanticipated conditions. Conducting prebid and preconstruction conferences can also be valuable in this respect.

Read Responsibility Provisions Closely

Some client representatives, design professionals, and constructors do not realize that geotechnical engineering is far less exact than other engineering disciplines. That lack of understanding has nurtured unrealistic expectations that have resulted in disappointments, delays, cost overruns, claims, and disputes. To confront that risk, geotechnical engineers commonly include explanatory provisions in their reports. Sometimes labeled "limitations," many of these provisions indicate where geotechnical engineers' responsibilities begin and end, to help others recognize their own responsibilities and risks. *Read these provisions closely*. Ask questions. Your geotechnical engineer should respond fully and frankly.

Geoenvironmental Concerns Are Not Covered

The personnel, equipment, and techniques used to perform an environmental study – e.g., a "phase-one" or "phase-two" environmental site assessment – differ significantly from those used to perform a geotechnical-engineering study. For that reason, a geotechnical-engineering report does not usually relate any environmental findings, conclusions, or recommendations; e.g., about the likelihood of encountering underground storage tanks or regulated contaminants. *Unanticipated subsurface environmental problems have led to project failures*. If you have not yet obtained your own environmental information, ask your geotechnical consultant for risk-management guidance. As a general rule, *do not rely on an environmental report prepared for a different client, site, or project, or that is more than six months old*.

Obtain Professional Assistance to Deal with Moisture Infiltration and Mold

While your geotechnical engineer may have addressed groundwater, water infiltration, or similar issues in this report, none of the engineer's services were designed, conducted, or intended to prevent uncontrolled migration of moisture – including water vapor – from the soil through building slabs and walls and into the building interior, where it can cause mold growth and material-performance deficiencies. Accordingly, *proper implementation of the geotechnical engineer's recommendations will not of itself be sufficient to prevent moisture infiltration*. Confront the risk of moisture infiltration by including building-envelope or mold specialists on the design team. *Geotechnical engineers are not building-envelope or mold specialists*.



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